

# Structural Design Services

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*Design calculations  
for  
The proposed internal opening  
at  
393 Bradford Road  
Huddersfield HD2 2QY*

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Rochdale  
OL11 2NN  
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3.6.24

## Note

*The following calculations are based on the plans & verbal instructions  
from the client. No structural or any other report is provided.*

*The contractor is to verify all the dimensions, the method of construction and any other  
queries with the designer before the start of the works and prior to  
ordering any building material .*

***Health & safety to be observed through out during the construction works.***

***No work to commence on site before the prior approval  
of the Building Regulation***

## Designed By

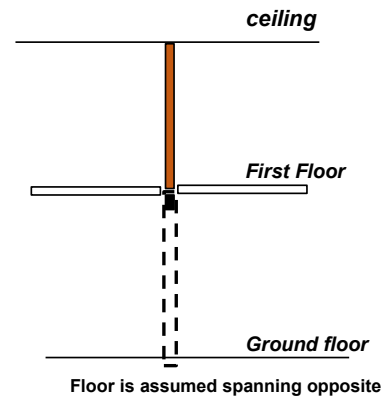
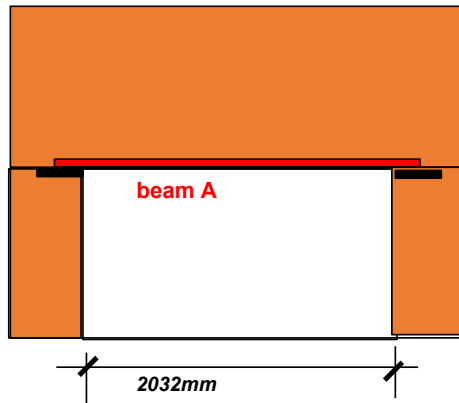
Nasir Hussain  
B Sc ( Civil Eng. )  
PE, MIE

## Designed For

***Syed Komail Zaidi***

## **Brief History**

Beam A is designed to carry the loadings above for the proposed works



All information is provided by the client

Beam A

Use 1 no.s 178 x 102 x19 UKB S355

use 100x 300 x 150mm deep concrete pad stone under the beam  
minimum bearing is 150mm

### **Note**

Please note that I have been instructed to design beam A  
so no comments on any other issue if there is any.  
All dimensions are to be verified on site

**Loadings**

		<i>service</i> <i>kn/m<sup>2</sup></i>	<i>gema</i> <i>m</i>	<i>ultimate</i> <i>kn/m<sup>2</sup></i>
<b>Roof</b>	Dead load = tiles,battens,rafters etc.	1	1.4	1.40
	Imposed load = roof + ceiling	0.75	1.6	1.2
	Total	<b>1.75</b>		<b>2.60</b>
<b>First floor</b>	Dead loads = boards,joists& ceilings	0.5	1.4	0.7
	Imposed loads =	1.5	1.6	2.4
	Total	<b>2</b>		<b>3.1</b>
<b>Gr. floor</b>	Dead load = 150mm conc. Slab	3.6	1.4	2.4*2
	Imposed loads =	1.5	1.6	2.4
	Total	<b>5.1</b>		<b>2.4</b>
<b>Ext.walls</b>	Dead load = 102mm Brickwork	2.1	1.4	2.94
	= 100mm Blockwork	1.8	1.4	2.52
	Total	<b>3.9</b>		<b>5.46</b>
<b>Intr.walls</b>	Dead load 100mm blockwall plaster both sides	2.1	1.4	2.94

## Steel Beam A

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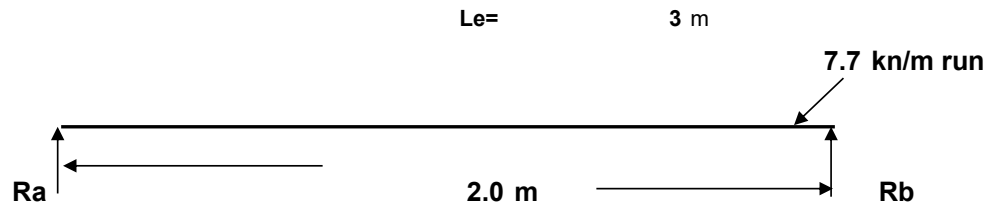
this proposed steel beam is supporting the following

1. Load from the Floors
2. Load from the wall
3. Load from the Roof
4. Self weight

### Loads

1. Load from the Floors=assumed spanning opposite	0 kn/m run
2. Load from the wall= $2.94 \times 2.5 =$	7 kn/m run
3. Load from the Roof=assumed no roof load shared by this beam	0 kn/m run
4. Self weight	0.3 kn/m run
	<hr/>
	7.7 kn/m run
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Span of steel beam = **2.0 m**



$$R_a = R_b = 7.8 \text{ kn}$$

$$\text{Moment} = 4 \text{ knm}$$

**Use 1 no.s 178 x 102 x 19 UKB S355**

$M_{bs} = 41.9 \text{ Knm}$        $M_b = 28.3 \text{ knm}$  for  $L_e = 3\text{m}$  and  $n = 1$   
plastic

## Deflection Check of Beam A

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Type of beam = simply supported

Type of loading = uniformly distributed load

Max.deflection is at mid span =  $5 FL^3/384EI$

where

$F = \text{total load on the beam} / 1.5 = 10.36 \text{ kn}$   
 $L = \text{span of the beam} = 2032 \text{ mm}$   
 $E = \text{elastic modulus of steel} = 205 \text{ kn/mm}^2$   
 $I = \text{moment of inertia of the beam} = 1360 \text{ cm}^4$

Actual deflec. = 0.4 mm

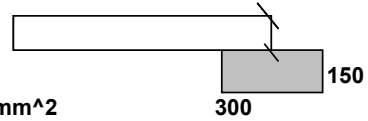
Allowable deflection = 6 mm (L / 360)  
> the actual

Therefore beam is ok in deflection

**Design Of Padstone Beam A**

Ultimate Point load =

7.8 kn



Mortar class ( iii ) 1 : 1 : 6

Blockwork Strength =

7 N/mm<sup>2</sup>

Compressive Strength =  $f_k$  =

6.4 N/mm<sup>2</sup>

from table 2a of BS 5628 part 1 1992

Partial safety factor =  $\gamma_m$  =

3.5

from table 4 of BS 5628 part 1 1992

Local design strength =  $1.5 \times f_k / \gamma_m$  =

2.74 N/mm<sup>2</sup>

Actual strength = ultimate load / area

Assuming size of padstone =

100 mm wide  
300 mm long  
150 mm deep concrete pad stone

Area =

30000 mm<sup>2</sup>

Actual stress =

0.26 N/mm<sup>2</sup>

As Actual stress is < design strength

therefore size of padstone assumed is ok

therefore use 100x 300 x 150mm deep concrete pad stone under the beam

minimum bearing is 150mm