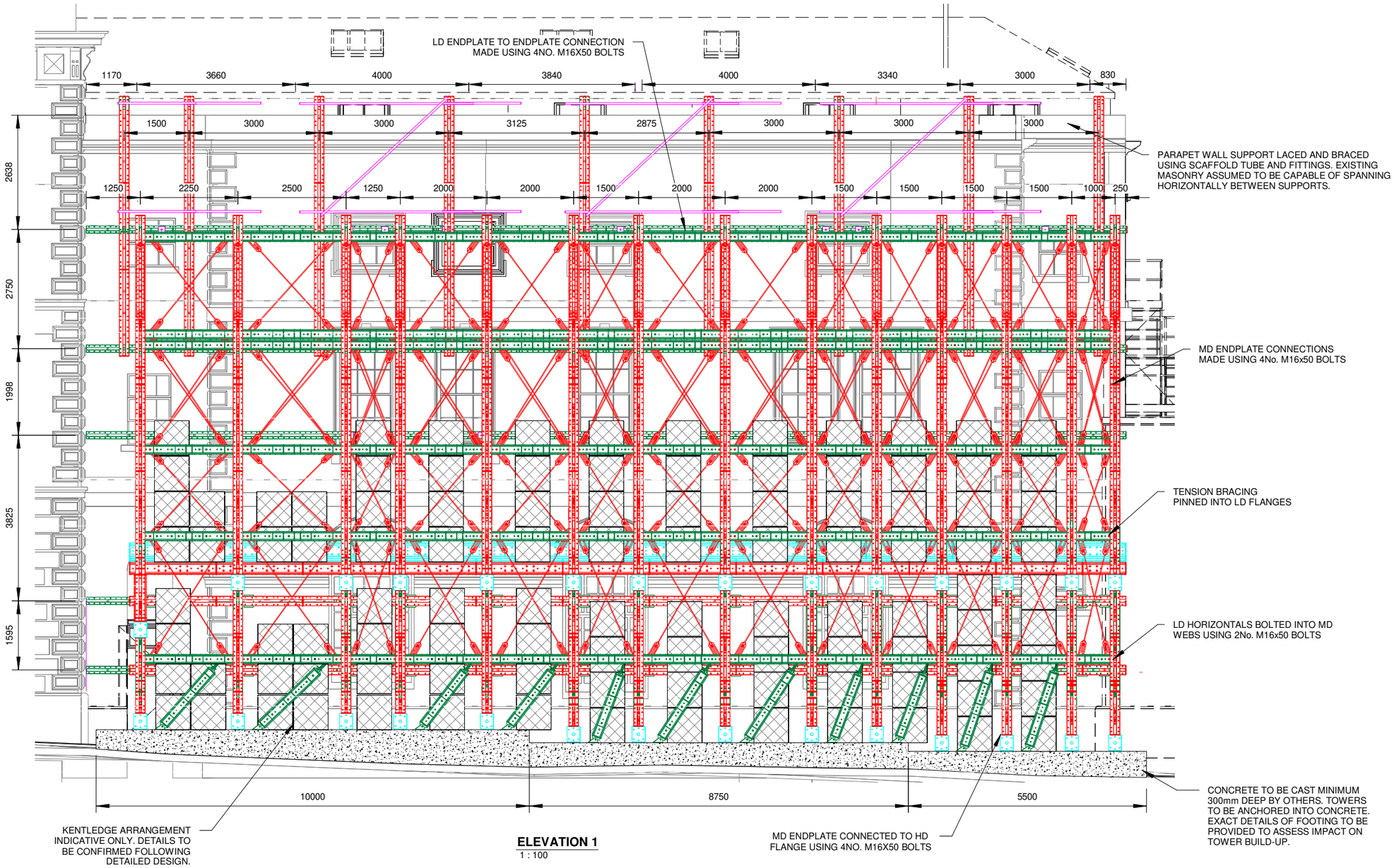

ELEVATION 1

1 : 100

KENTLEDGE

Total kentledge required approx. 320Te. Exact details and placement TBC following detailed design.

PRELIMINARY ISSUE

Preliminary drawings issued for discussion only.

TEMPORARY BASE

Maximum SLS leg loads to be resisted by the temporary base are 231kN compression and 48kN shear. Weight of concrete base used as kentledge, weight calculated based on dimensions shown and a minimum thickness of 300mm.

INTERNAL WALERS

Due to the complexity of the new steel facade support frame, it is not possible to break the walers around the new vertical members. It is recommended that a phased install/transfer of load from the temporary works to the new structure is recommended. Details TBC.

DO NOT SCALE FROM THIS DRAWING

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DESIGN REFERENCE: Rev

DRAWING NUMBERING

100 Series - Layouts / IDID Drawings
200 Series - Sections
300 Series - Details
500 Series - Fabrication Drawings

SECTION & DETAIL NOTATION

201A — Denotes Section A on drawing 201
301B — Denotes Detail B on drawing 301

GENERAL NOTES

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DESIGN NOTES

A. Basis of Design
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brief72859rev1_v2
GHH-BOW-GA-ZZ-M-A-0001 The George Hotel-Building A S2 - Information P11
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B. Deflection
The maximum deflection of the facade restraint system is anticipated to be less than 18mm (<H/750). Customer to ensure this is acceptable.

C. Foundations
MGF have estimated the size of the foundations / kentledge required to resist sliding and overturning. The design of the foundation to safely react to the loads from the tower is the responsibility of the customer. An estimated bearing pressure of 80kN/m² is anticipated to be imposed by the temporary foundation.

D. Facade
It is the responsibility of the customer to check the capacity of the facade to span between points of restraint. MGF cannot accept responsibility for the assessment of the state or condition of the facade.

RESIDUAL RISKS

1. Foundations / Support
Where MGF equipment is supported / anchored / tied to an existing structure or ground, the Customer must ensure that the existing structure or ground is adequate to safely support the imposed loads. This design has not made any allowance for differential settlement of supports / foundations.
2. Anchors / Fixings
The design and supply of anchors and fixings between the MGF system and the temporary footing are the responsibility of the customer. The customer must ensure suitable fixings to resist the imposed loads as detailed.
3. Eccentricity
The customer must ensure that all legs are erected plumb and loaded concentrically. For general tolerances this design considers a maximum total eccentricity of 25mm in one axis. If this is exceeded on site the customer must contact MGF immediately.
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The customer must ensure stability during erection and dismantling of the temporary supports. Additional temporary supports, struts and tubes may be required to aid stability during erection & dismantling. It is the customer's responsibility to produce a method statement for the safe installation and removal of the temporary supports.
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Suitable edge protection and means of safe access & egress must be provided.
7. Deflections
The customer must check and advise of any deflection limitations prior to commencing works. Excessive deflection can lead to structural cracks and damage to the existing brickwork / masonry.
8. Packing
Where packing is required, the customer is to ensure it is adequate to transfer the load effectively.
9. Existing Wall
It is the customers responsibility to ensure the existing wall is capable of spanning between needles / supports / restraints. The customer must verify this prior to commencing works.
10. Lateral Deflections (Facades)
A maximum theoretical deflection of 13mm has been calculated. The deflection criteria of Height / 750 has been adopted in accordance with CIRIA guidance document C579. The customer must ensure this is acceptable for the works being undertaken.
11. Loose Masonry
All loose or damaged masonry must be removed. The customer must be aware of the risk of loose or damaged masonry falling between needles / supports / restraints. Suitable controls must be in place to mitigate risks to operatives.
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14. Global Stability
The integrity and global stability of the existing structure is the customer's responsibility. This has not been considered as part of the temporary works design. The customer should monitor the existing structure for signs of movement. The customer should satisfy themselves that the existing structure is in a condition that will not lead to damage of the existing structure during the temporary condition.

Issue	Issue Description	Date	Designed	Drawn	Checked
P2	Support to parapet wall added. Wall tie projection reduced.	25.01.24	Jagger Gledhill	Jagger Gledhill	Stephen Barker
P1	Preliminary Issue	21.11.23	Jagger Gledhill	Jagger Gledhill	Stephen Barker
T1	Issued for discussion only	07.11.23	Mia Charnock	Molly Barnes	Stephen Barker

PRELIMINARY

STRUCTURAL SUPPORT SOLUTIONS
MGF Design Services Ltd.
Foundation House, Wallwork Road,
Astley, Manchester, M29 7JT
design@mgf.co.uk | mgf.co.uk | 01942 402 704

Customer: MORGAN SINDALL CONSTRUCTION INFRASTRUCTURE

Site: St George's Square, Huddersfield, HD1 1JA

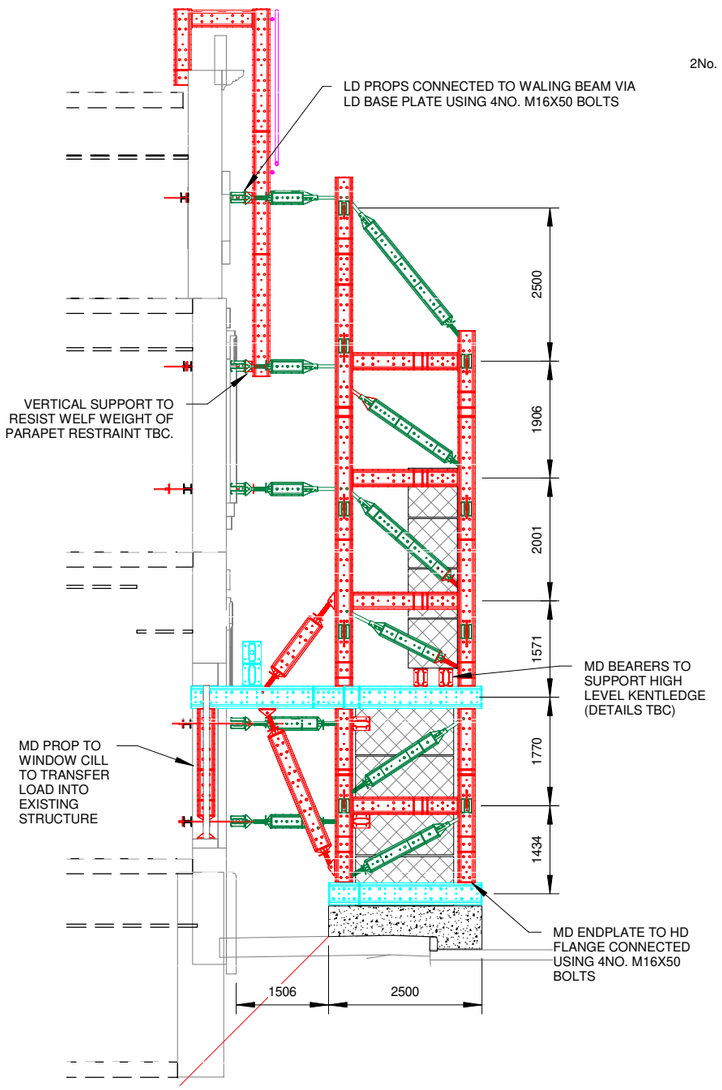
Scheme: George Hotel Façade Retention

Drawing Title: Elevation

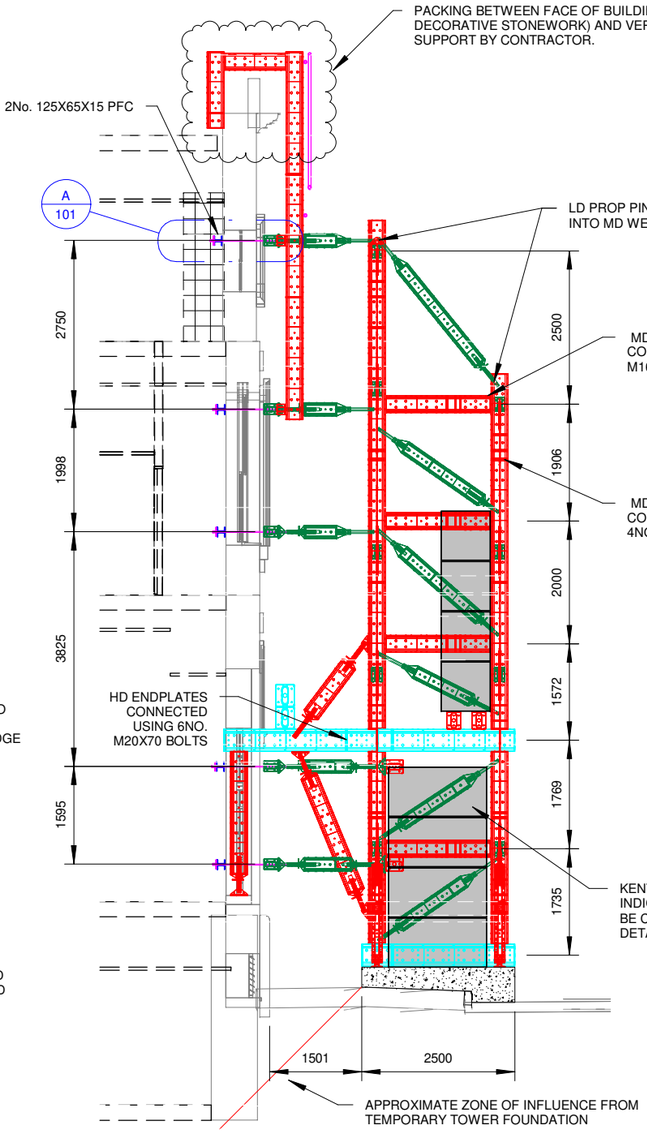
Date: 07.11.23 | Designed: MC | Drawn: MB | Checked: SB

Drawing No: 72859 - 100 | Rev: P2

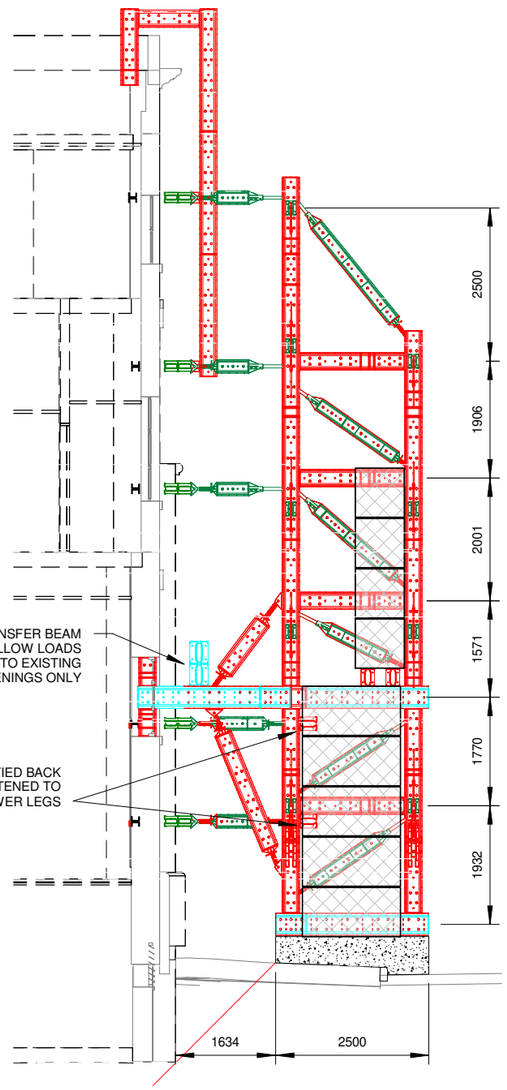
A0 Sheet



SECTION A
1 : 100



SECTION B
1 : 100



SECTION C
1 : 100

KENTLEDGE

Total kentledge required approx. 320Te. Exact details and placement TBC following detailed design.

TEMPORARY BASE

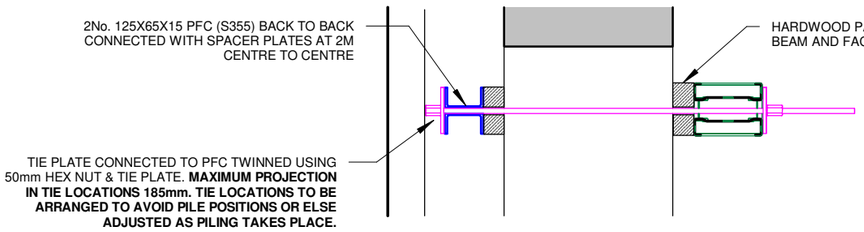
Maximum SLS leg loads to be resisted by the temporary base are 231kN compression and 48kN shear. Weight of concrete base used as kentledge, weight calculated based on dimensions shown and a minimum thickness of 300mm.

PRELIMINARY ISSUE

Preliminary drawings issued for discussion only.

INTERNAL WALERS

Due to the complexity of the new steel facade support frame, it is not possible to break the walers around the new vertical members. It is recommended that a phased install/transfer of load from the temporary works to the new structure is recommended. Details TBC.



DETAIL A - WALING BEAMS TIED THROUGH FACADE
1 : 20

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100 Series - Layouts / IDID Drawings
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SECTION & DETAIL NOTATION

201A Denotes Section A on drawing 201
301B Denotes Detail B on drawing 301

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B. Deflection
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P2	Support to parapet wall added. Wall tie projection reduced.	25.01.24	Jagger Gledhill	Jagger Gledhill	Stephen Barker
P1	Preliminary Issue	21.11.23	Jagger Gledhill	Jagger Gledhill	Stephen Barker
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PRELIMINARY

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MGF Design Services Ltd.
Foundation House, Wallwork Road,
Astley, Manchester, M29 7JT
design@mgf.co.uk | mgf.co.uk | 01942 402 704

Customer
MORGAN SINDALL CONSTRUCTION INFRASTRUCTURE

Site
St George's Square, Huddersfield, HD1 1JA

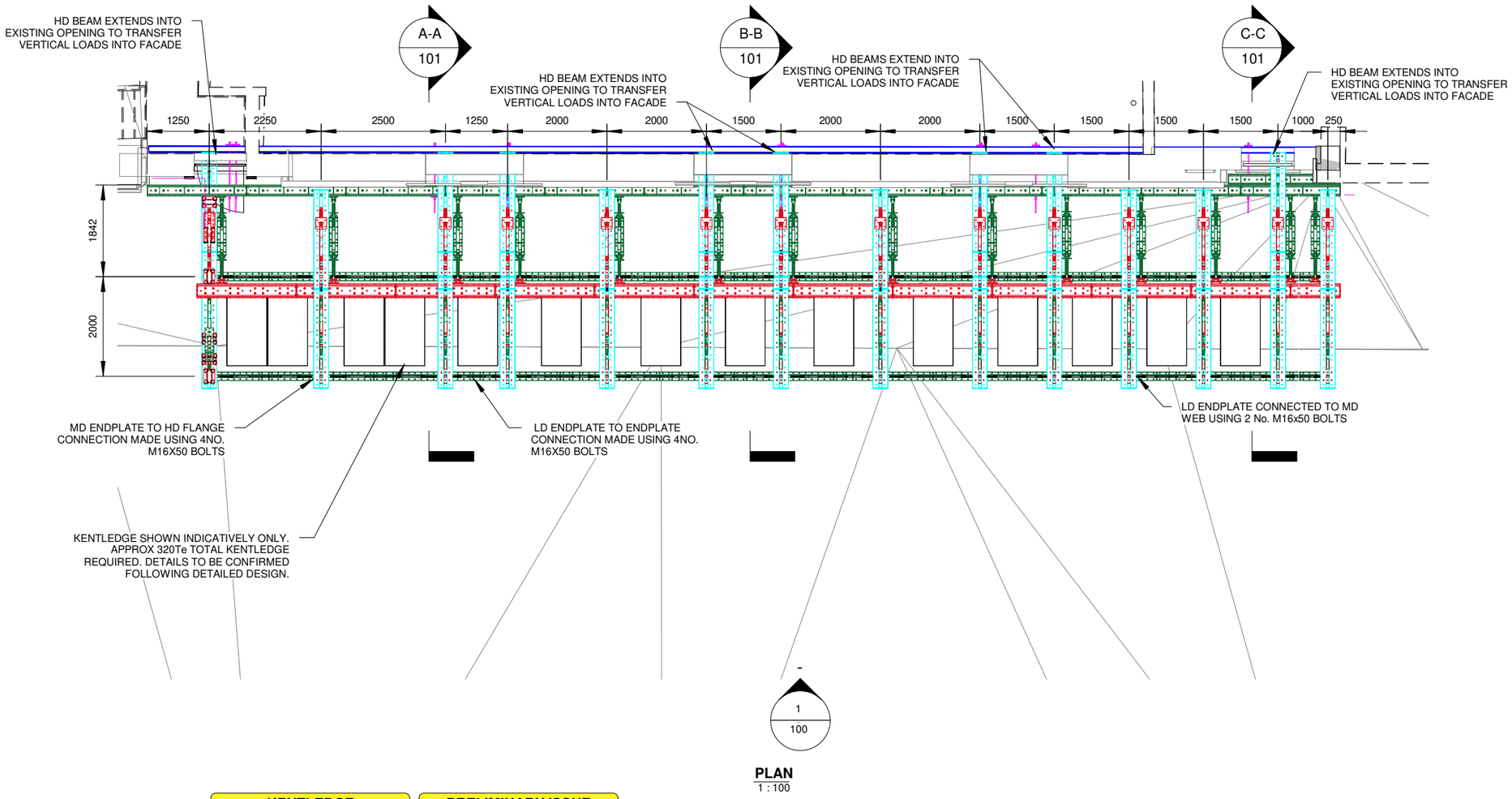
Scheme
George Hotel Façade Retention

Drawing Title
Section A & Detailing

Date: 07.11.23 **Designed:** MC **Drawn:** MB **Checked:** SB

Drawing No 72859 - 101 **Rev** P2

A3 Sheet



PLAN
1 : 100

KENTLEDGE
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Customer: MORGAN SINDALL CONSTRUCTION INFRASTRUCTURE

Site: St George's Square, Huddersfield, HD1 1JA

Scheme: George Hotel Façade Retention

Drawing Title: Plan

Date: 07.11.23 | Designed: MC | Drawn: MB | Checked: SB

A3 Sheet

Drawing No: 72859 - 102 | Rev: P2