

**DO NOT SCALE FROM THIS DRAWING**

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DESIGN REFERENCE:		Rev
<b>DRAWING NUMBERING</b>	<b>SECTION &amp; DETAIL NOTATION</b>	
100 Series - Layouts / IDID Drawings	201A	Denotes Section A on drawing 201
200 Series - Sections		
300 Series - Details	301B	Denotes Detail B on drawing 301
500 Series - Fabrication Drawings		

- GENERAL NOTES**
- All dimensions are in (mm). Levels in (m). Weights in (kg).
  - This drawing should NOT be scaled from. This drawing should be read in conjunction with a relevant site specific, Safe System of Work (SSoW).
  - For further information on MGF products visit [www.mgf.co.uk](http://www.mgf.co.uk)
  - MGF recommend that the temporary works are inspected before each working shift by a competent person. (Normally the temporary works coordinator or supervisor).

**DESIGN NOTES**

- A. Basis of Design**  
This design has been prepared from information supplied to MGF by, or on behalf of the Customer (ref. below), who must ensure requirements have been correctly interpreted.  
brief72859rev1\_v2  
GH-BOW-GA-ZZ-M-A-0001 The George Hotel-Building A S2 - Information P11  
Where available, this drawing should be read in conjunction with the MGF Design Reference as noted below.  
Equipment detailed has been verified in accordance with the MGF Technical File (Technical Guidance for UniShore) available for download from our website. For the latest information on MGF products please visit [www.mgf.co.uk](http://www.mgf.co.uk)
- B. Deflection**  
The maximum deflection of the facade restraint system is anticipated to be less than 18mm (<H/750). Customer to ensure this is acceptable.
- C. Foundations**  
MGF have estimated the size of the foundations / kentledge required to resist sliding and overturning. The design of the foundation to safely react to the loads from the tower is the responsibility of the customer. An estimated bearing pressure of 800kN/m<sup>2</sup> is anticipated to be imposed by the temporary foundation.
- D. Facade**  
It is the responsibility of the customer to check the capacity of the facade to span between points of restraint. MGF cannot accept responsibility for the assessment of the state or condition of the facade.

**RESIDUAL RISKS**

- Foundations / Support**  
Where MGF equipment is supported / anchored / tied to an existing structure or ground, the Customer must ensure that the existing structure or ground is adequate to safely support the imposed loads. This design has not made any allowance for differential settlement of supports / foundations.
- Anchors / Fixings**  
The design and supply of anchors and fixings between the MGF system and the temporary footing are the responsibility of the customer. The customer must ensure suitable fixings to resist the imposed loads as detailed.
- Eccentricity**  
The customer must ensure that all legs are erected plumb and loaded concentrically. For general tolerances this design considers a maximum total eccentricity of 25mm in one axis. If this is exceeded on site the customer must contact MGF immediately.
- Installation & Removal**  
The customer must ensure stability during erection and dismantling of the temporary supports. Additional temporary supports, struts and tubes may be required to aid stability during erection & dismantling. It is the customer's responsibility to produce a method statement for the safe installation and removal of the temporary supports.
- Bracing**  
The stability of the temporary works is dependent on bracing being installed. The system must not be loaded until all detailed bracing is installed. The installation sequence should be carefully considered so adequate bracing is in situ as installation progresses.
- Working At Height / Safe Access & Egress**  
Suitable edge protection and means of safe access & egress must be provided.
- Deflections**  
The customer must check and advise of any deflection limitations prior to commencing works. Excessive deflection can lead to structural cracks and damage to the existing brickwork / masonry.
- Packing**  
Where packing is required, the customer is to ensure it is adequate to transfer the load effectively.
- Existing Wall**  
It is the customer's responsibility to ensure the existing wall is capable of spanning between needs / supports / restraints. The customer must verify this prior to commencing works.
- Lateral Deflections (Facades)**  
A maximum theoretical deflection of 13mm has been calculated. The deflection criteria of Height / 750 has been adopted in accordance with C191A guidance document C573. The customer must ensure this is acceptable for the works being undertaken.
- Loose Masonry**  
All loose or damaged masonry must be removed. The customer must be aware of the risk of loose or damaged masonry falling between needs / supports / restraints. Suitable controls must be in place to mitigate risks to operatives.
- Kentledge**  
The supply and location of kentledge is the responsibility of the customer. The customer must ensure kentledge provides a restoring load greater than the load stated on the drawing.
- Accidental Impact**  
The customer must ensure suitable controls are in place to prevent impact / collisions with the temporary works. No allowance has been made for loading due to accidental impact. MGF should be provided with any anticipated accidental impact load cases during demolition and erection of the new building frame.
- Global Stability**  
The integrity and global stability of the existing structure is the customer's responsibility. This has not been considered as part of the temporary works design. The customer should monitor the existing structure for signs of movement. The customer should satisfy themselves that the existing structure is in a condition that will not lead to damage of the existing structure during the temporary condition.

P2	Support to parapet wall added. Wall tie projection reduced.	25.01.24	Jagger Gledhill	Jagger Gledhill	Stephen Barker
P1	Preliminary Issue	21.11.23	Jagger Gledhill	Jagger Gledhill	Stephen Barker
T1	Issued for discussion only	07.11.23	Mia Charnock	Molly Barnes	Stephen Barker
Issue	Issue Description	Date	Designed	Drawn	Checked

**PRELIMINARY**

**STRUCTURAL SUPPORT SOLUTIONS**

**MGF**

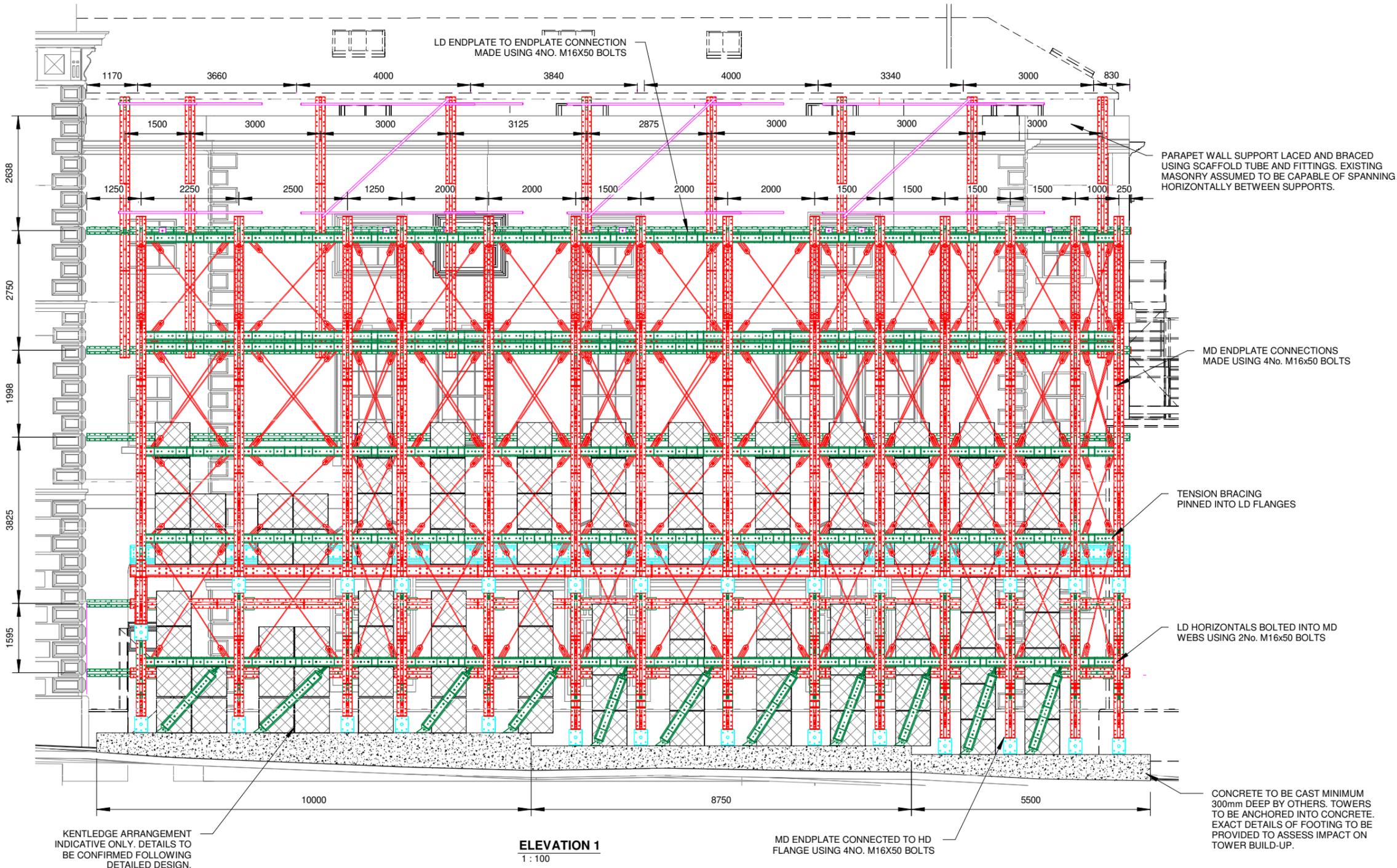
MGF Design Services Ltd.  
Foundation House, Wallwork Road,  
Astley, Manchester, M29 7JT  
design@mgf.co.uk | mgf.co.uk | 01942 402 704

Customer **MORGAN SINDALL**  
CONSTRUCTION INFRASTRUCTURE

Site St George's Square, Huddersfield, HD1 1JA

Scheme George Hotel Façade Retention

Drawing Title		Elevation	
Date: 07.11.23	Designed: MC	Drawn: MB	Checked: SB
Drawing No	72859 - 100		Rev
			P2



**ELEVATION 1**  
1 : 100

KENTLEDGE ARRANGEMENT INDICATIVE ONLY. DETAILS TO BE CONFIRMED FOLLOWING DETAILED DESIGN.

<p><b>KENTLEDGE</b></p> <p>Total kentledge required approx. 320Te. Exact details and placement TBC following detailed design.</p>	<p><b>PRELIMINARY ISSUE</b></p> <p>Preliminary drawings issued for discussion only.</p>
<p><b>TEMPORARY BASE</b></p> <p>Maximum SLS leg loads to be resisted by the temporary base are 231kN compression and 48kN shear. Weight of concrete base used as kentledge, weight calculated based on dimensions shown and a minimum thickness of 300mm.</p>	<p><b>INTERNAL WALERS</b></p> <p>Due to the complexity of the new steel facade support frame, it is not possible to break the walers around the new vertical members. It is recommended that a phased install/transfer of load from the temporary works to the new structure is recommended. Details TBC.</p>

A3 Sheet

**DO NOT SCALE FROM THIS DRAWING**

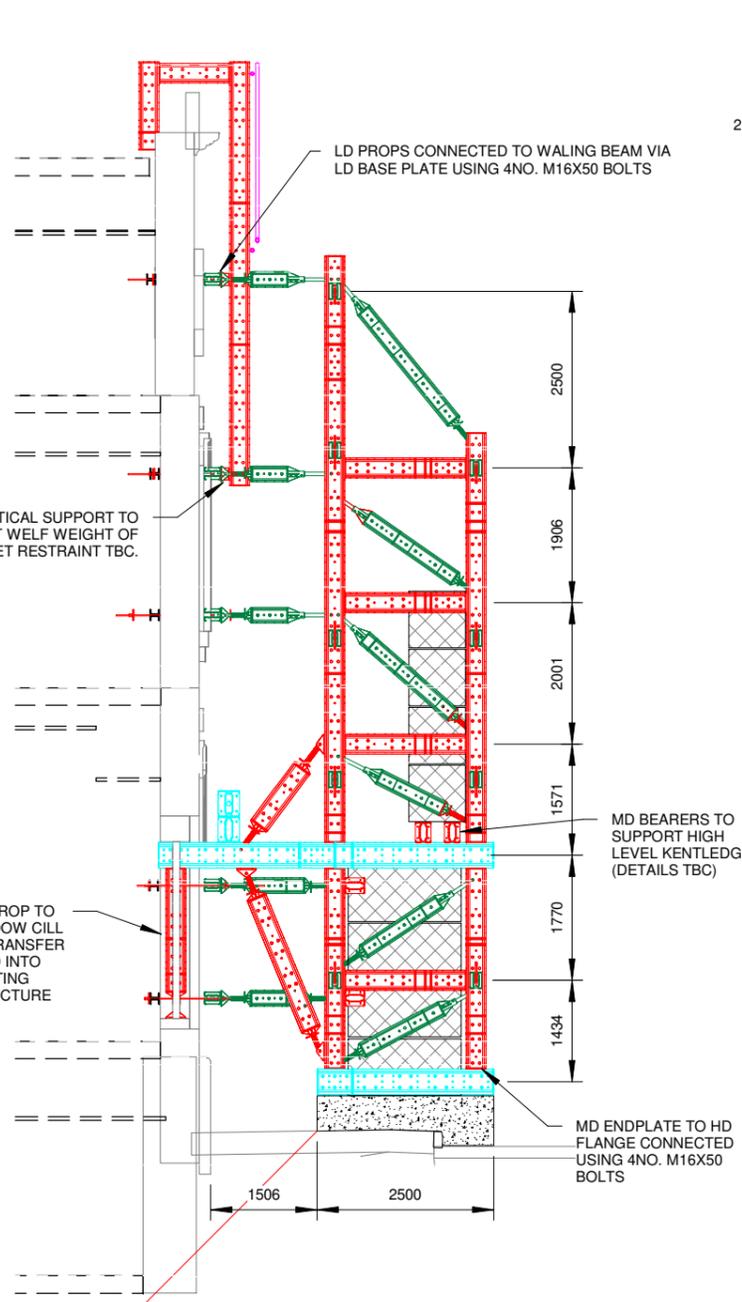
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DESIGN REFERENCE:		Rev
DRAWING NUMBERING	SECTION & DETAIL NOTATION	
100 Series - Layouts / IDID Drawings	201A	Denotes Section A on drawing 201
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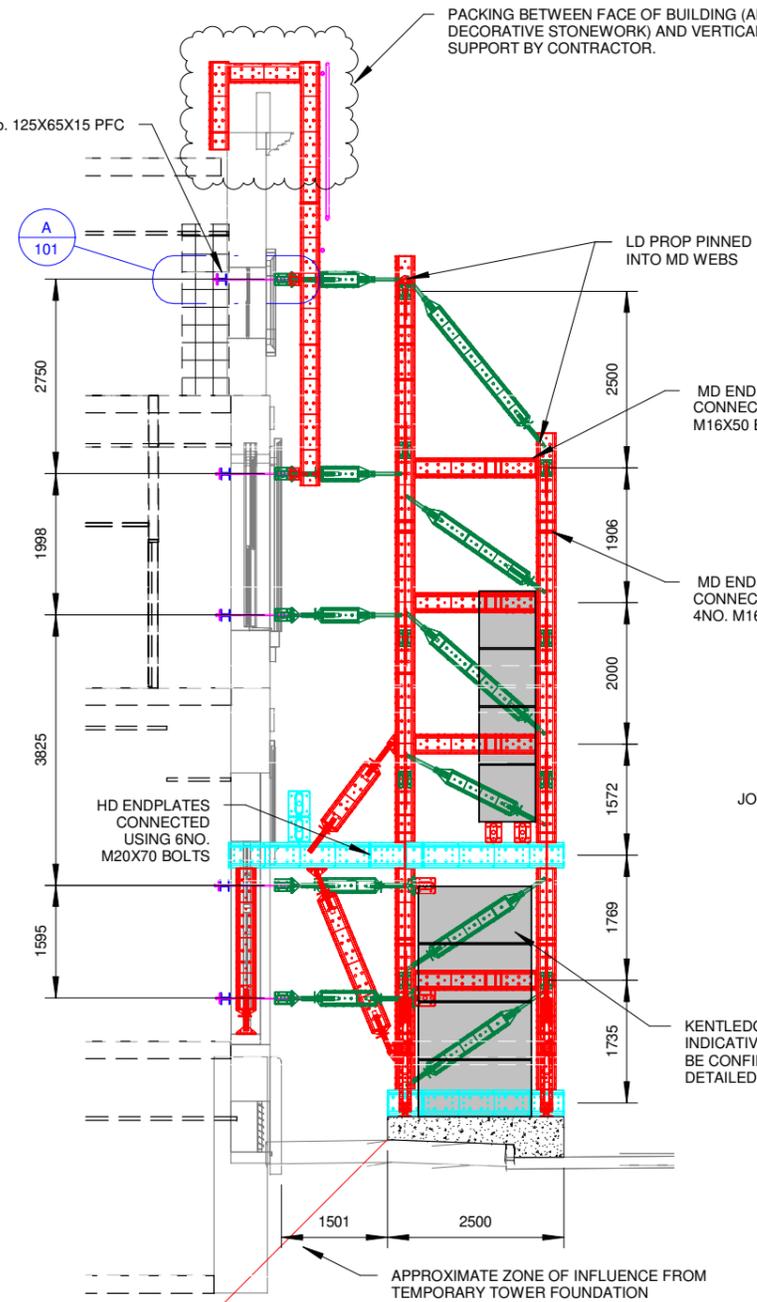
- GENERAL NOTES**
- All dimensions are in (mm), Levels in (m), Weights in (kg).
  - This drawing should NOT be scaled from. This drawing should be read in conjunction with a relevant site specific, Safe System of Work (SSoW).
  - For further information on MGF products visit [www.mgf.co.uk](http://www.mgf.co.uk)
  - MGF recommend that the temporary works are inspected before each working shift by a competent person. (Normally the temporary works coordinator or supervisor).

- DESIGN NOTES**
- A. **Basis of Design**  
This design has been prepared from information supplied to MGF by, or on behalf of the Customer (ref. below), who must ensure requirements have been correctly interpreted. [brief72859rev1\\_v2](#)  
G14-BOW-GA-22-M-A-0001 The George Hotel-Building A S2- Information P11  
Where available, this drawing should be read in conjunction with the MGF Design Reference as noted below.  
Equipment detailed has been verified in accordance with the MGF Technical File (Technical Guidance for UnShore) available for download from our website. For the latest information on MGF products please visit [www.mgf.co.uk](http://www.mgf.co.uk)
- B. **Deflection**  
The maximum deflection of the facade restraint system is anticipated to be less than 18mm (<H/750). Customer to ensure this is acceptable.
- C. **Foundations**  
MGF have estimated the size of the foundations / kentledge required to resist sliding and overturning. The design of the foundation to safely react to the loads from the tower is the responsibility of the customer. An estimated bearing pressure of 100kN/m<sup>2</sup> is anticipated to be imposed by the temporary foundation.
- D. **Facade**  
It is the responsibility of the customer to check the capacity of the facade to span between points of restraint. MGF cannot accept responsibility for the assessment of the state or condition of the facade.

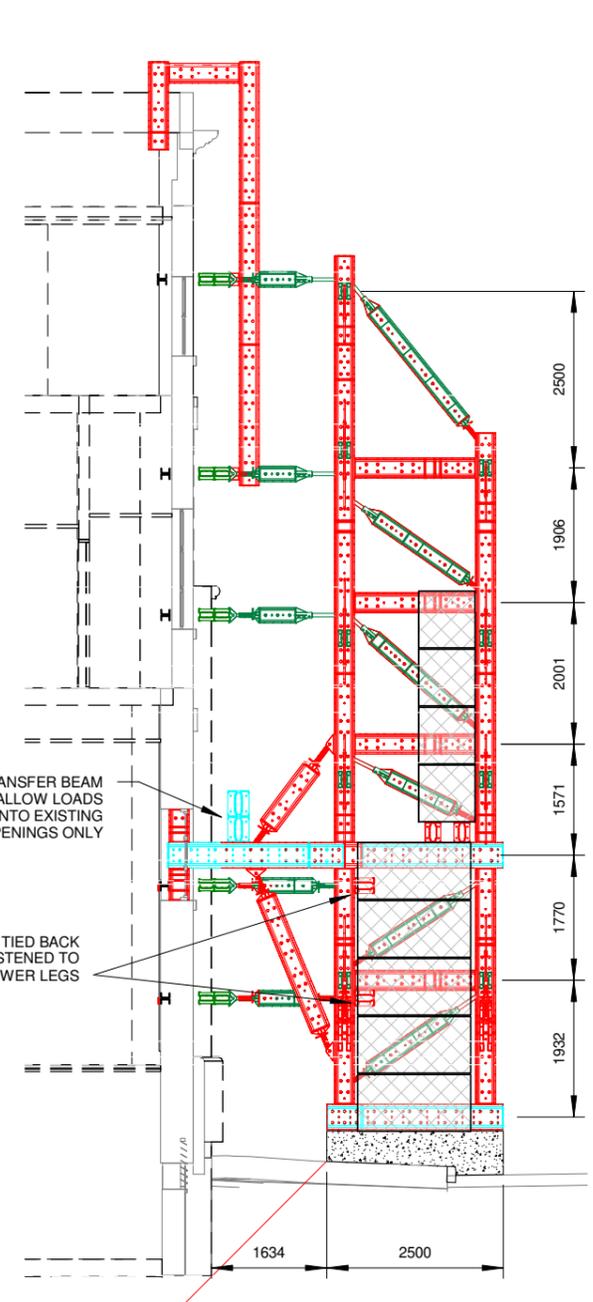
- RESIDUAL RISKS**
- Foundations / Support**  
Where MGF equipment is supported / anchored / tied to an existing structure or ground, the Customer must ensure that the existing structure or ground is adequate to safely support the imposed loads. This design has not made any allowance for differential settlement of supports / foundations.
  - Anchors / Fixings**  
The design and supply of anchors and fixings between the MGF system and the temporary footing are the responsibility of the customer. The customer must ensure suitable fixings to resist the imposed loads as detailed.
  - Erection / Dismantling**  
The customer must ensure that all legs are erected plumb and loaded concentrically. For general tolerances this design considers a maximum total eccentricity of 25mm in one axis. If this is exceeded on site the customer must contact MGF immediately.
  - Installation & Removal**  
The customer must ensure stability during erection and dismantling of the temporary supports. Additional temporary supports, struts and tubes may be required to aid stability during erection & dismantling. It is the customer's responsibility to produce a method statement for the safe installation and removal of the temporary supports. Bracing  
The stability of the temporary works is dependent on bracing being installed. The system must not be loaded until all detailed bracing is installed. The installation sequence should be carefully considered so adequate bracing is in situ as installation progresses.
  - Working At Height / Safe Access & Egress**  
Suitable edge protection and means of safe access & egress must be provided.
  - Deflections**  
The customer must check and advise of any deflection limitations prior to commencing works. Excessive deflection can lead to structural cracks and damage to the existing brickwork / masonry.
  - Packing**  
Where packing is required, the customer is to ensure it is adequate to transfer the load effectively.
  - Existing Wall**  
It is the customer's responsibility to ensure the existing wall is capable of spanning between needies / supports / restraints. The customer must verify this prior to commencing works.
  - Lateral Deflections (Facades)**  
A maximum theoretical deflection of 13mm has been calculated. The deflection criteria of Height / 750 has been adopted in accordance with CIRIA guidance document C579. The customer must ensure this is acceptable for the works being undertaken.
  - Loose Masonry**  
All loose or damaged masonry must be removed. The customer must be aware of the risk of loose or damaged masonry falling between needies / supports / restraints. Suitable controls must be in place to mitigate risks to operatives.
  - Kentledge**  
The supply and location of kentledge provides a restoring load greater than the load stated on the drawing.
  - Accidental Impact**  
The customer must ensure suitable controls are in place to prevent impact / collisions with the temporary works. No allowance has been made for loading due to accidental impact. MGF should be provided with any anticipated accidental impact load cases during demolition and erection of the new building frame.
  - Global Stability**  
The integrity and global stability of the existing structure is the customer's responsibility. This has not been considered as part of the temporary works design. The customer should monitor the existing structure for signs of movement. The customer should satisfy themselves that the existing structure is in a condition that will not lead to damage of the existing structure during the temporary condition.



**SECTION A**  
1 : 100



**SECTION B**  
1 : 100



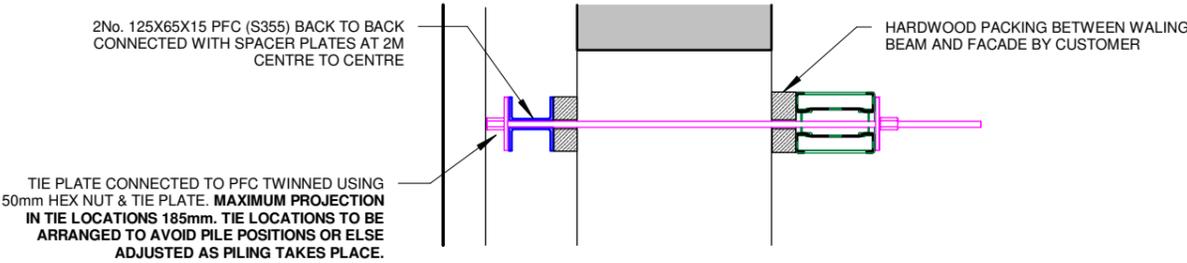
**SECTION C**  
1 : 100

**KENTLEDGE**  
Total kentledge required approx. 320Te. Exact details and placement TBC following detailed design.

**TEMPORARY BASE**  
Maximum SLS leg loads to be resisted by the temporary base are 231kN compression and 48kN shear. Weight of concrete base used as kentledge, weight calculated based on dimensions shown and a minimum thickness of 300mm.

**PRELIMINARY ISSUE**  
Preliminary drawings issued for discussion only.

**INTERNAL WALERS**  
Due to the complexity of the new steel facade support frame, it is not possible to break the walers around the new vertical members. It is recommended that a phased install/transfer of load from the temporary works to the new structure is recommended. Details TBC.



**DETAIL A - WALING BEAMS TIED THROUGH FACADE**  
1 : 20

**PRELIMINARY**

**STRUCTURAL SUPPORT SOLUTIONS**

**MGF**  
MGF Design Services Ltd.  
Foundation House, Wallwork Road,  
Astley, Manchester, M29 7JT  
[design@mgf.co.uk](mailto:design@mgf.co.uk) | [mgf.co.uk](http://mgf.co.uk) | 01942 402 704

Customer: **MORGAN SINDALL**  
CONSTRUCTION INFRASTRUCTURE

Site: St George's Square, Huddersfield, HD1 1JA

Scheme: George Hotel Façade Retention

Drawing Title: Section A & Detailing

Date: 07.11.23	Designed: MC	Drawn: MB	Checked: SB
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Drawing No: **72859 - 101** Rev: **P2**

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brief:Z859rev1\_v2  
GH4-BOW-GA-ZZ-M-A-0001 The George Hotel-Building A S2 - Information P11  
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  - Deflection**  
The maximum deflection of the facade restraint system is anticipated to be less than 18mm (-H750). Customer to ensure this is acceptable.
  - Foundations**  
MGF have estimated the size of the foundations / kentledge required to resist sliding and overturning. The design of the foundation to safely react to the loads from the tower is the responsibility of the customer. An estimated bearing pressure of 80kN/m<sup>2</sup> is anticipated to be imposed by the temporary foundation.
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  - Packing**  
Where packing is required, the customer is to ensure it is adequate to transfer the load effectively.
  - Existing Wall**  
It is the customer's responsibility to ensure the existing wall is capable of spanning between needles / supports / restraints. The customer must verify this prior to commencing works.
  - Lateral Deflections / Facades**  
A maximum theoretical deflection of 13mm has been calculated. The deflection criteria of Height / 750 has been adopted in accordance with CIRIA guidance document C579. The customer must ensure this is acceptable for the works being undertaken.
  - Loose Masonry**  
All loose or damaged masonry must be removed. The customer must be aware of the risk of loose or damaged masonry falling between needles / supports / restraints. Suitable controls must be in place to mitigate risks to operatives.
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P2	Support to parapet wall added. Wall tie projection reduced.	25.01.24	Jagger Gledhill	Jagger Gledhill	Stephen Barker
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T1	Issued for discussion only	07.11.23	Mia Charnock	Molly Barnes	Stephen Barker
Issue	Issue Description	Date	Designed	Drawn	Checked

**PRELIMINARY**

**STRUCTURAL SUPPORT SOLUTIONS**

**MGF**  
MGF Design Services Ltd.  
Foundation House, Wallwork Road,  
Astley, Manchester, M29 7JT  
[design@mgf.co.uk](mailto:design@mgf.co.uk) | [mgf.co.uk](http://mgf.co.uk) | 01942 402 704

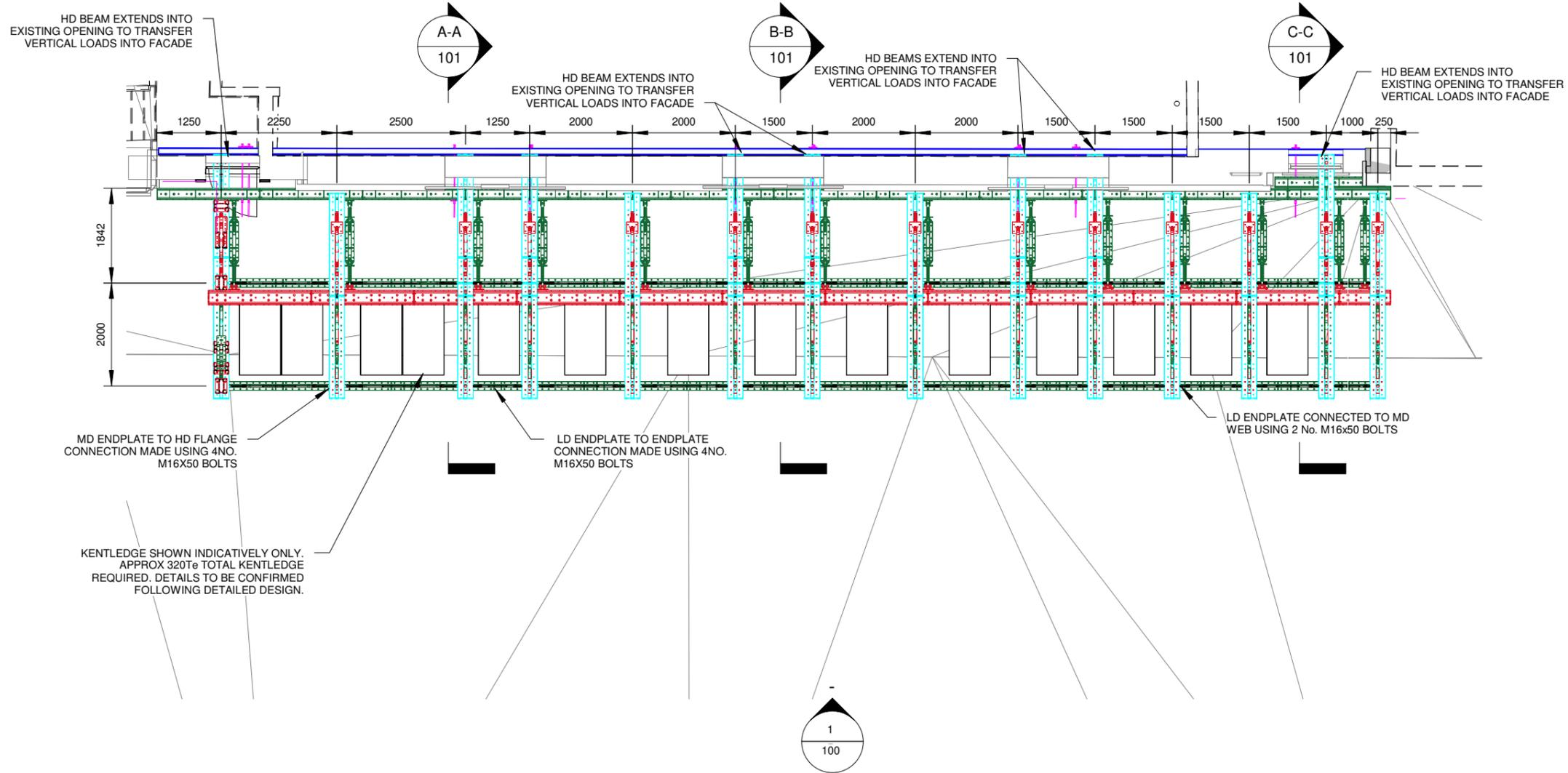
**Customer**  
**MORGAN SINDALL**  
CONSTRUCTION INFRASTRUCTURE

**Site**  
St George's Square, Huddersfield, HD1 1JA

**Scheme**  
George Hotel Façade Retention

**Drawing Title**  
Plan

Date: 07.11.23	Designed: MC	Drawn: MB	Checked: SB
Drawing No	72859 - 102		Rev
			P2



MD ENDPLATE TO HD FLANGE CONNECTION MADE USING 4NO. M16X50 BOLTS

LD ENDPLATE TO ENDPLATE CONNECTION MADE USING 4NO. M16X50 BOLTS

LD ENDPLATE CONNECTED TO MD WEB USING 2 No. M16X50 BOLTS

KENTLEDGE SHOWN INDICATIVELY ONLY. APPROX 320T<sub>e</sub> TOTAL KENTLEDGE REQUIRED. DETAILS TO BE CONFIRMED FOLLOWING DETAILED DESIGN.

**KENTLEDGE**  
Total kentledge required approx. 320T<sub>e</sub>. Exact details and placement TBC following detailed design.

**TEMPORARY BASE**  
Maximum SLS leg loads to be resisted by the temporary base are 231kN compression and 48kN shear. Weight of concrete base used as kentledge, weight calculated based on dimensions shown and a minimum thickness of 300mm.

**PRELIMINARY ISSUE**  
Preliminary drawings issued for discussion only.

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Due to the complexity of the new steel facade support frame, it is not possible to break the walers around the new vertical members. It is recommended that a phased install/transfer of load from the temporary works to the new structure is recommended. Details TBC.

**PLAN**  
1 : 100