

Gabion analysis

Input data

Project

Task : Stability at Cellars Clough
 Part : Section AA
 Description : Gabion Wall 1+2+3+4 Section
 Author : Mark Richardson
 Date : 08/01/2020
 Project ID : 6018 CCZ

Material of blocks - filling

No.	Name	g [kN/m ³]	φ [°]	c [kPa]
1	Material No. 1	18.00	44.00	0.00

Material of blocks - mesh

No.	Name	Strength overh. R _t [kN/m]	Spacing of vert. meshes v [m]	Bear.cap. of front joint R _s [kN/m]
1	Material No. 1	20.00	1.00	20.00

Soil parameters

MADE GROUND

Unit weight : g = 20.00 kN/m³
 Stress-state : effective
 Angle of internal friction : j_{ef} = 44.00 °
 Cohesion of soil : c_{ef} = 0.00 kPa
 Angle of friction struc.-soil : d = 30.00 °
 Soil : cohesionless
 Saturated unit weight : g_{sat} = 20.00 kN/m³

CLAY 1

Unit weight : g = 20.00 kN/m³
 Stress-state : effective
 Angle of internal friction : j_{ef} = 33.00 °
 Cohesion of soil : c_{ef} = 0.00 kPa
 Angle of friction struc.-soil : d = 25.00 °
 Soil : cohesionless
 Saturated unit weight : g_{sat} = 20.00 kN/m³

Backfill

Assigned soil : MADE GROUND
 Slope = 45.00 °

Foundation

Type of foundation : strip foundation
 Unit weight of foundation g = 25.00 kN/m³

Geometry of foundation

Foundation thickness h = 0.60 m
 Offset left b_l = 0.15 m
 Offset right b_p = 0.15 m

Parameters of contact base-soil

Friction coefficient f = 0.577
 Cohesion c = 0.00 kPa

Additional resistance $F = 0.00$ kN/m

Water influence

Ground water table is located below the structure.

Input surface surcharges

No.	Surcharge		Action	Mag.1 [kN/m ²]	Mag.2 [kN/m ²]	Ord.x x [m]	Length l [m]	Depth z [m]
	new	change						
1	Yes		permanent	84.00	45.00	3.10	2.30	on terrain
No.	Name							
1	Wall 2 surcharge							

Resistance on front face of the structure

Resistance on front face of the structure: at rest

Soil on front face of the structure - MADE GROUND

Soil thickness in front of structure $h = 0.10$ m

Terrain in front of structure is flat.

Verification No. 1

Verification of complete wall

Check for overturning stability

Resisting moment $M_{res} = 98.40$ kNm/m

Overturning moment $M_{ovr} = 15.22$ kNm/m

Wall for overturning is SATISFACTORY

Check for slip

Resisting horizontal force $H_{res} = 49.58$ kN/m

Active horizontal force $H_{act} = 29.74$ kN/m

Wall for slip is SATISFACTORY

Overall check - WALL is SATISFACTORY

Maximum stress in footing bottom : 44.39 kPa

Bearing capacity of foundation soil

Verification of foundation soil

Stress in the footing bottom : trapezoid

Eccentricity verification

Max. eccentricity of normal force $e = 0.036$

Maximum allowable eccentricity $e_{alw} = 0.333$

Eccentricity of the normal force is SATISFACTORY

Verification of bearing capacity

Max. stress at footing bottom $s = 76.98$ kPa

Bearing capacity of foundation soil $R_d = 100.00$ kPa

Bearing capacity of foundation soil is SATISFACTORY

Overall verification - bearing capacity of found. soil is SATISFACTORY

Slope stability analysis

Input data

Surcharge

No.	Type	Type of action	Location z [m]	Origin x [m]	Length l [m]	Width b [m]	Slope a [°]	Magnitude		
								q, q ₁ , f, F	q ₂	unit
1	trapezoid	permanent	on terrain	x = 3.10	l = 2.30		0.00	84.00	45.00	kN/m ²
2	trapezoid	permanent	on terrain	x = 7.34	l = 2.30		0.00	84.00	45.00	kN/m ²
3	trapezoid	permanent	on terrain	x = 10.58	l = 2.30		0.00	84.00	45.00	kN/m ²

Surcharges

No.	Name
1	Wall 2 surcharge
2	Wall 3 surcharge
3	Wall 4 surcharge

Results (Stage of construction 1)

Analysis 1

Slope stability verification (Bishop)

Combination 1

Sum of active forces : $F_a = 652.88$ kN/m

Sum of passive forces : $F_p = 956.81$ kN/m

Sliding moment : $M_a = 8820.36$ kNm/m

Resisting moment : $M_p = 12926.52$ kNm/m

Utilization : 68.2 %

Slope stability ACCEPTABLE

Combination 2

Sum of active forces : $F_a = 544.54$ kN/m

Sum of passive forces : $F_p = 697.37$ kN/m

Sliding moment : $M_a = 7356.69$ kNm/m

Resisting moment : $M_p = 9421.52$ kNm/m

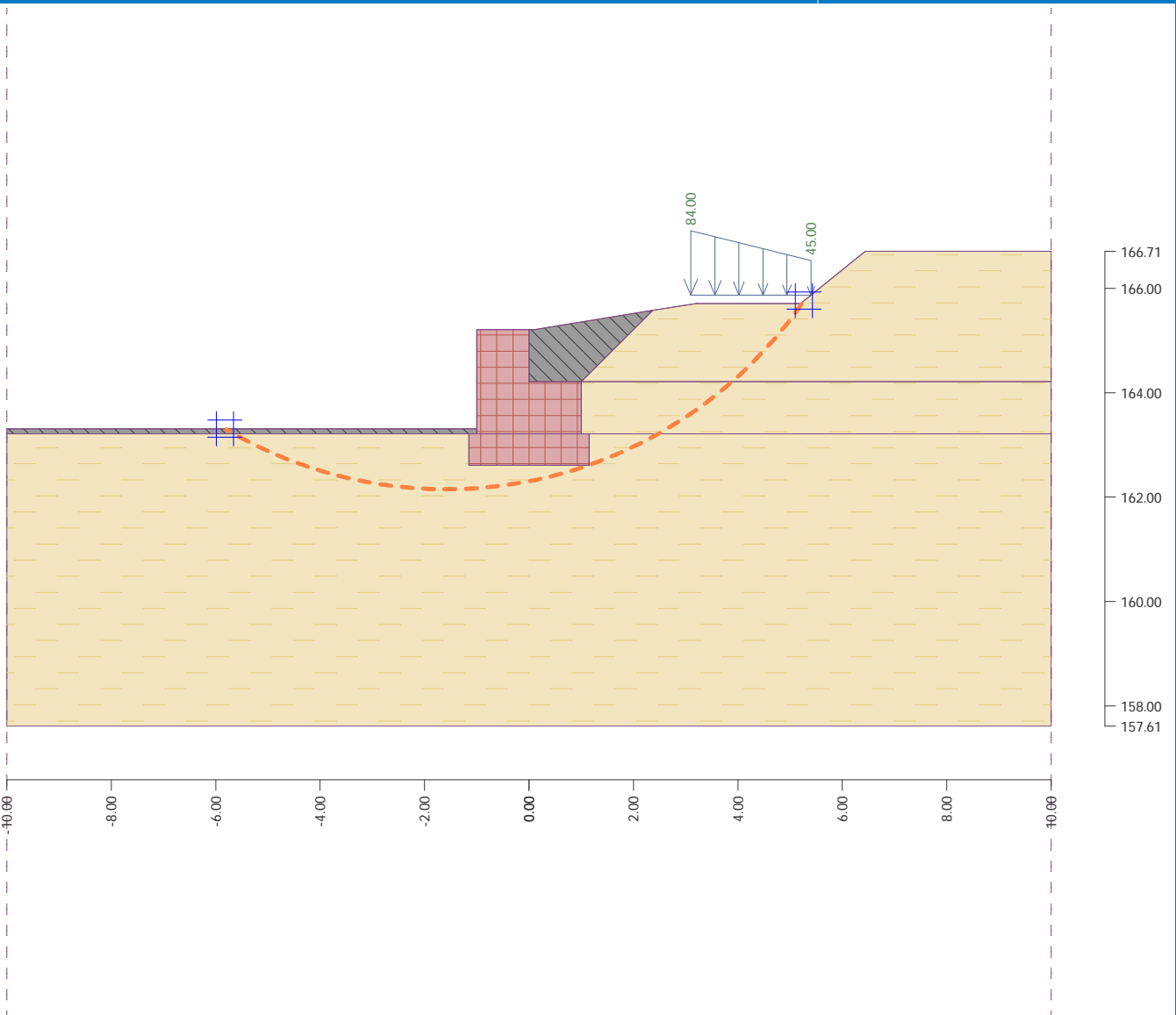
Utilization : 78.1 %

Slope stability ACCEPTABLE

Optimized slip surface for : Combination 2

Name :

Stage - analysis : 1 - 1



The slip surface after optimization.

Slope stability verification (Bishop)

Combination 1

Sum of active forces : $F_a = 235.61$ kN/m

Sum of passive forces : $F_p = 324.61$ kN/m

Sliding moment : $M_a = 1957.95$ kNm/m

Resisting moment : $M_p = 2697.54$ kNm/m

Utilization : 72.6 %

Slope stability ACCEPTABLE

Combination 2

Sum of active forces : $F_a = 185.17$ kN/m

Sum of passive forces : $F_p = 228.48$ kN/m

Sliding moment : $M_a = 1538.73$ kNm/m

Resisting moment : $M_p = 1898.70$ kNm/m

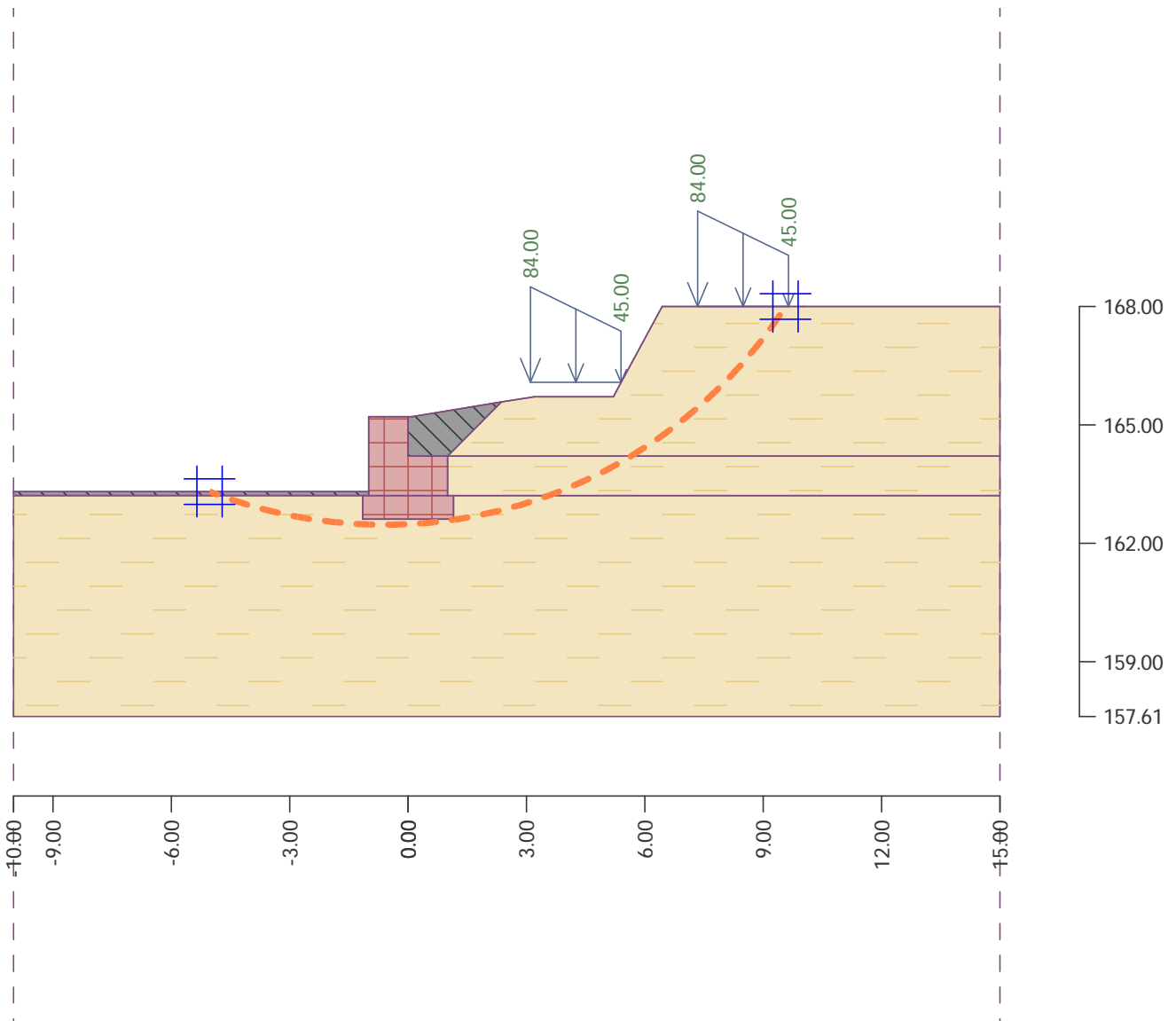
Utilization : 81.0 %

Slope stability ACCEPTABLE

Optimized slip surface for : Combination 2

Name :

Stage - analysis : 1 - 1



The slip surface after optimization.

Slope stability verification (Bishop)

Combination 1

Sum of active forces : $F_a = 406.79 \text{ kN/m}$

Sum of passive forces : $F_p = 581.75 \text{ kN/m}$

Sliding moment : $M_a = 4926.22 \text{ kNm/m}$

Resisting moment : $M_p = 7044.96 \text{ kNm/m}$

Utilization : 69.9 %

Slope stability ACCEPTABLE

Combination 2

Sum of active forces : $F_a = 336.19 \text{ kN/m}$

Sum of passive forces : $F_p = 420.05 \text{ kN/m}$

Sliding moment : $M_a = 4071.26 \text{ kNm/m}$

Resisting moment : $M_p = 5086.80 \text{ kNm/m}$

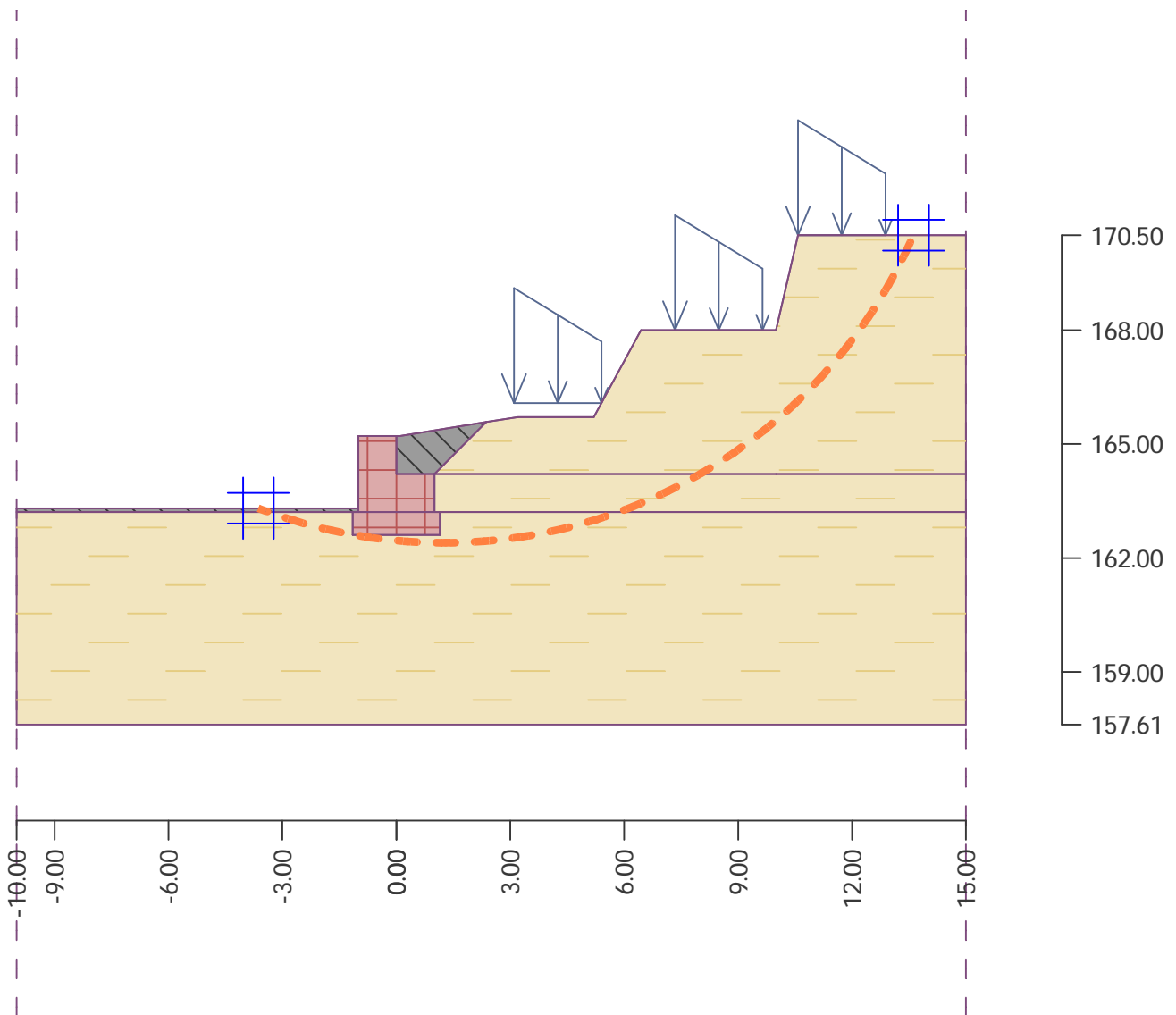
Utilization : 80.0 %

Slope stability ACCEPTABLE

Optimized slip surface for : Combination 2

Name :

Stage - analysis : 1 - 1



The slip surface after optimization.

Slope stability verification (Bishop)

Combination 1

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 Sum of passive forces : $F_p = 956.81$ kN/m
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Slope stability ACCEPTABLE

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