

SYNGENTA, HUDDERSFIELD

SURFACE WATER DRAINAGE STATEMENT

Project name **Syngenta Huddersfield – Flood Risk Review**
 Project no. **1620015210**
 Client **Syngenta UK Ltd**
 Memo no. **REH2022N02505-RAM-ME-00003**
 Version **0.1**

Date 06/09/2023

1 Introduction

Ramboll UK Ltd ('Ramboll') has been commissioned by Syngenta Ltd ('the Client') to provide a Surface Water Drainage Statement for planning permission sought for the erection of a New Warehouse, Storeroom and Charging Room on an existing industrial site at Syngenta Huddersfield Manufacturing Centre, HD5 9TE.

Ramboll
 1 Broad Gate
 The Headrow
 Leeds
 LS1 8EQ
 United Kingdom

2 Kirklees Statutory Drainage Requirements

The statutory requirements outlined in the West Yorkshire Combined Authority (WYCA) SuDS Guidance document¹ has been used to determine the required scope and level of detailed for this report. An email consultation response received from Kirklees Council in November 2022 has also been used to establish the reporting requirements and strategy approach.

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<https://uk.ramboll.com/environment-and-health>

3 Proposed Development

A summary of the nature of each aspect of the development assessed within this note is included below:

- **Warehouse:** This warehouse will be used as a storage facility for dry powders held in large flexible bags, this warehouse will contain storage racking;
- **New Storeroom:** This storeroom will be used as a storage and warming facility for a liquid raw material, held in drums, this storeroom will contain racking; and
- **New Charging Room:** This charging room will be used to charge liquid raw material from drums into the main process plant.

The location of each of the above are presented in Figure 1 below, adapted from drawing 4/7901 12 provided by the Client.

¹ www.kirklees.gov.uk/beta/flooding-and-drainage/pdf/sustainable-urban-drainage.pdf

Ramboll UK Limited
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 Registered office:
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effluent treatment system. Typically, the rainwater will be held in this tank for 6-12 hours prior to discharge.

Effluent Treatment System

The site operates an extensive dual wastewater drainage system with very large capacity as it is designed and consented to handle and discharge substantially higher volumes of effluent than it does currently. Surface water drains for rainwater exist in areas of no industrial operations, and are discharged to the river Colne. The majority of the drains on site, including all of those in the development area (including for roof water), are routed to the sites dedicated water storage and treatment facility. Once treated, this water is discharged to a series of Yorkshire Water treatment plants prior to final discharge into the River Calder.

The site effluent treatment plant has four containment tanks which hold a total volume of 12,000 m³. Typically, 5,000 m³ of water is handled per day. It takes approximately 12 hours from inlet to outlet at the effluent treatment plant, and the outlet flow is at a controlled rate so heavy rainfall events do not lead to substantial increase in loading on the Yorkshire Water treatment works. If the site is under threat of flooding, all chemical operations are shut down to minimise the load on the drainage systems. A schematic diagram of the process for both rainwater collection and effluent treatment is included in Figure 1 below.

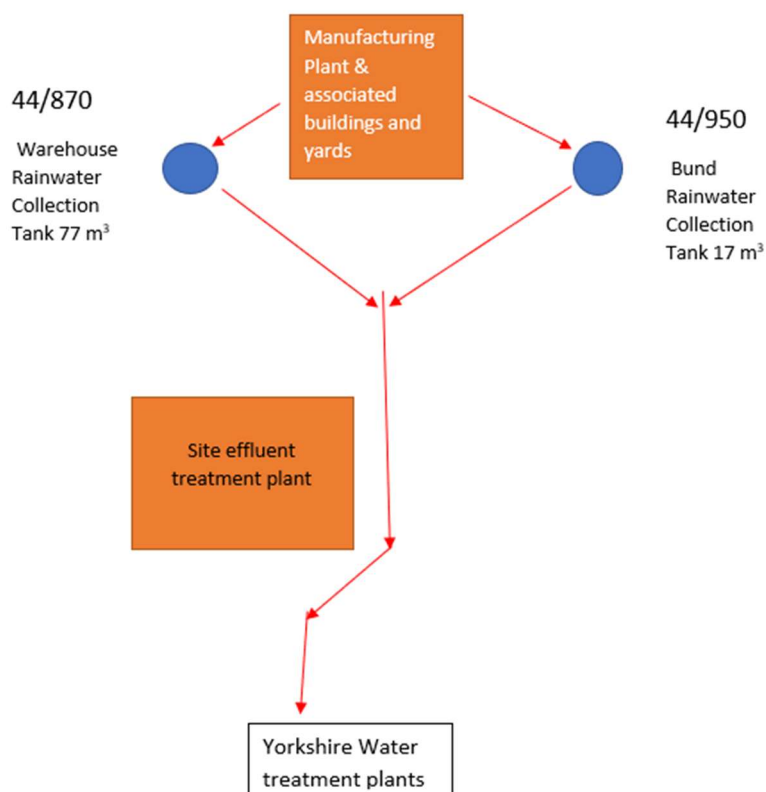


Figure 1 – Schematic of current rainwater collection and effluent treatment system

5 Proposed Runoff and Storage

A consultation response received from Kirklees Council in November 2022 stated that due to the length of time since the previous buildings were demolished, a greenfield rate of runoff would be more suitable than brownfield. A greenfield rate of runoff for the 1 in 1 year storm event has therefore been used in the calculations below, which were undertaken with the Micro Drainage modelling software using the 'Source Control' module. This greenfield rate was used to calculate the required storage volume for the 0.057 ha impermeable area attenuating the 1 in 100 year plus 40% climate change event.

Table 1 – Runoff and Storage Calculations

Impermeable area (ha)	Greenfield runoff Rate (l/s)	Required Storage (m ³)	Critical Storm	Peak Rainfall Intensity (mm/hr)
0.057	0.3	43	960 min, Winter	7.16

A copy of the Micro Drainage calculations sheet is included in Appendix B.

6 Proposed Drainage

The WYCA SuDS guidance document states that disposal of surface water should be done so according to the discharge hierarchy stated in Building Regulations Part H, which requires infiltration to ground and discharge to a watercourse to be considered prior to discharge to a sewer. In consideration of the current precautionary approach that is taken to manage runoff (which may potentially be contaminated), discharge via infiltration or directly to the River Colne is not considered appropriate in this instance.

Instead, and in line with the existing site-wide system for capturing, treating, storing and discharging surface water, any runoff generated by the proposed development will be collected into the existing rainwater capture system, where it is tested and attenuated for around 6-12 hours. From here, rainwater is discharged to the effluent treatment system. All contributing drains and gullies will discharge to a sump, which will then transferred to the treatment system. Surface water discharge from the treatment system will vary, and can be reduced/stopped at the request of Yorkshire Water.

7 Summary

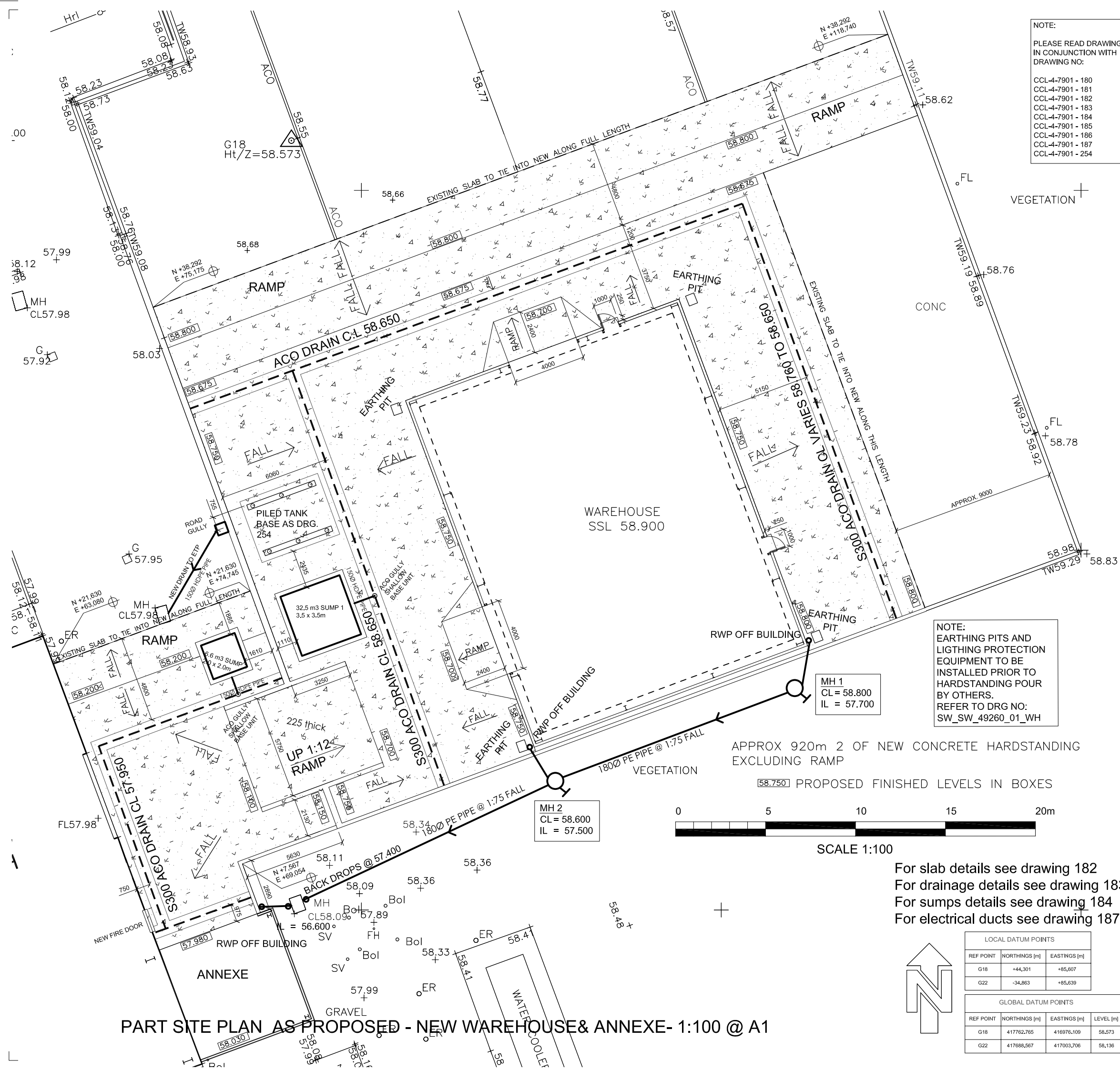
The development proposals are for the introduction of a Warehouse, Storeroom and Charging Room within the southern extent of the existing Syngenta site. The development will not result in the introduction of any impermeable area, but will be housed on existing slabs and areas of hardstanding.

At the request of Kirklees Council, a greenfield rate of runoff has been used to identify level of required storage. To attenuate to 0.3 l/s (the 1 in 100 year plus 40% climate change event), a total of 43 m³ of storage would be required.

The method for treating and storing surface water will retain the existing system in place, whereby runoff is collected into the central effluent system where it undergoes treatment for removal of any contaminants prior to being discharged at a controlled rate to an existing Yorkshire Water sewer, and then to the River Colne. Effluent from the system discharges at a controlled rate over a 24 hour period, and discharge can also be shut off completely if required.

The proposed strategy above ensures that, in line with the WYCA SuDS guidance, there is no detriment to water quality or quantity resulting from the development, and that surface water can be managed in a safe and sustainable manner.

APPENDIX A – DEVELOPMENT PROPOSALS



NOTE:
PLEASE READ DRAWING
IN CONJUNCTION WITH
DRAWING NO:
CCL-4-7901 - 180
CCL-4-7901 - 181
CCL-4-7901 - 182
CCL-4-7901 - 183
CCL-4-7901 - 184
CCL-4-7901 - 185
CCL-4-7901 - 186
CCL-4-7901 - 187
CCL-4-7901 - 254

NOTES

THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT PM PROJENS AND ENGINEERS DRAWINGS AND SPECIFICATIONS

ALL WORK TO BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE HEALTH & SAFETY AT WORK ACT 1974 AND THE CONSTRUCTION [DESIGN & MANAGEMENT] REGULATIONS 2015

ALL LEVELS ARE IN METRES AND UNLESS NOTED OTHERWISE ARE STRUCTURAL LEVELS RELATED TO ORDNANCE DATUM

CONTRACTOR TO VISIT SITE TO ASSESS SITE RESTRICTIONS, ACCESS ISSUES ETC. PRIOR TO TENDERING.

DO NOT SCALE OFF THIS DRAWING. ALL DETAILS AND DIMENSIONS ARE TO BE CHECKED BY THE CONTRACTOR PRIOR TO COMMENCEMENT OF CONSTRUCTION. ANY DISCREPANCIES ARE TO BE REPORTED TO THE ENGINEER.

ROADWAY / RAMP SPECIFICATION

MIN. 200 THK. CONCRETE TO SLAB TO BE GRADE RC40 FIBRE REINFORCED - ONE LAYER OF A252 MESH IN BOTTOM POLYPROPYLENE FIBRES BY SIKA OR SIMILAR APPROVED

NOMINAL SIZE OF AGGREGATE SHALL NOT EXCEED 20MM.

RAMP TO BE MIN 225 THICK AS DETAILED WITH UPSTANDS TO SIDES WITH ARMCO BARRIERS - SEE DRAWING 182

SUB-BASE SHOULD BE CONSTRUCTED USING A GRANULAR MATERIAL WHICH CONFORMS TO TYPE 1 GRADING STANDARDS

SUB-BASE SHALL BE PLACED IN LAYERS NOT EXCEEDING 150mm IN THICKNESS AND SHALL BE COMPACTED TO ACHIEVE 95% OF ITS MAX DRY DENSITY. AT THE OPTIMUM MOISTURE CONTENT (+/- 2%) AS SPECIFIED IN BS1377-4: 1990

CONCRETE. BLINDED AND 1200 g VISQUEEN DPM - EXPOSED CONCRETE EDGES AT JOINTS TO BE FINISHED WITH A 20MM. BULL NOSED TROWEL. ALL JOINT SEALANTS TO BE RECESSED 3MM. FROM FINISHED PAVING LEVELS.

SLAB FINISH TAMPED / HEAVY BRUSH TO PROVIDE GRIP FOR VEHICLES. TROWELLED MARGINS TO ALL JOINT EDGES

PREPRUFE MEMBRANE UNDER ALL NEW CONCRETE ALONG WITH WATERBARS TO ALL JOINTS

BURN OFF EXCESSIVE FIBRES BROUGHT TO SURFACE

REINFORCEMENT TO BE IN ACCORDANCE WITH BS. 4449. ALL REINFORCEMENT TO BE GRADE 500 N/mm².

MIN LAPS
H10 BARS TO BE 400 mm

MIN. 400 mm LAPS TO MESH

MIN 40mm COVER TO ALL REINFORCEMENT

SAW CUTS TO SLAB TO BE DONE BETWEEN 8N° - 24N° HOURS AFTER POURING CONC.

WELDED FOSROC WATERBARS TO ALL JOINTS IN SLAB

BAR REINFORCEMENT TO COMPLY WITH BS 4449 & BS 8666.

MESH REINFORCEMENT TO COMPLY WITH BS 4449 OR BS 4482/BS 4483.

FINISHED SURFACES TO UPSTANDS BE TYPE A IN ACCORDANCE WITH BS 8110: PART 1: 1997 CLAUSE 6.2.7.3.

ALL EXPOSED CONCRETE EDGES TO PLINTHS TO HAVE A 25 X 25 CHAMFER.

CONCRETE CUBE TESTS TO BE TAKEN IN ACCORDANCE WITH SPECIFICATION

ALL SEALANTS etc. TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS / RECOMMENDATIONS

SLABS SHOULD BE SAWN AS SOON AS THE SURFACE IS CAPABLE OF ACCEPTING THE OPERATION WITHOUT DAMAGE.

CONTRACTOR TO PROPOSE JOINTING ARRANGEMENT FOR APPROVAL TO SUIT PROGRAMME AND SITE LOGISTICS

CONTRACTOR TO ALLOW FOR JOINTS AND DETAILS AS DRAWING 182

DRAINAGE NOTES:

CONTRACTOR TO FULLY COMPLY WITH SITE SPECIFIC HEALTH AND SAFETY PLAN

ALL DIMENSIONS ARE IN MILLIMETRES UNLESS NOTED OTHERWISE

CONTRACTOR TO PROVIDE ALL NECESSARY METHOD STATEMENTS AND RISK ASSESSMENTS FOR PROJEN APPROVAL AND OBTAIN ALL APPROPRIATE WORK PERMITS

EXISTING SERVICES AND PIPEWORK RECORD DRAWINGS ARE AVAILABLE ON SITE. CONTRACTOR TO ENSURE AREA IS CLEAR OF BELOW GROUND SERVICES PRIOR TO CARRYING OUT WORKS

C1	16/12/22	ISSUED FOR CONSTRUCTION	JT	JA	CY
Rev	26/05/22	TENDER	CY	JA	CY
Date		Description	By	Check	App.

EXISTING DRAINAGE LAYOUT AND SITE LEVELS ON THIS DRAWING ARE TO BE CONFIRMED BY THE CONTRACTOR ON SITE PRIOR TO COMMENCEMENT OF DRAINAGE WORKS. THE ENGINEER IS TO BE NOTIFIED OF ANY DISCREPANCIES.

ALL NEW PIPES UP TO BE 180Ø POLYETHYLENE (PE100 grade) BLACK SOLID WALL (10mm) WITH FUSION COUPLINGS UNO

FOR STANDARD MANHOLE AND PIPE BEDDING DETAILS REFER TO DRAWING 183
DRAINAGE CHANNELS TO BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS

EXISTING MANHOLES, LEVELS & PIPE SIZES TO BE CHECKED ON SITE
INVERT LEVELS OF EXISTING PIPES TO BE CHECKED PRIOR TO INSTALLATION OF NEW DRAINAGE SYSTEM TO ENSURE GRAVITY SYSTEM IS FEASIBLE

ALL OUTGOING AND INCOMING PIPES AT MANHOLE JUNCTIONS TO BE CONNECTED INVERT TO INVERT UNLESS SPECIFIED OTHERWISE.

ALL PIPES TO BE ENCASED IN MIN 150 CONCRETE

NEW CHANNEL DRAINS TO BE ACO S300 INSTALLED WITH LOCKABLE HD SLOTTED DUCTILE IRON COVERS (D400 RATED) INCORPORATING ALL NECESSARY PROPRIETARY ACCESSORIES.

900 mm DP SUMPS TO BE TRAPPED AND RODDABLE.

CHANNELS ENCASED IN MIN. 200 mm BED AND SURROUND C20/C25 CONCRETE

CHANNELS INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURERS DETAILS AND RECOMMENDED SPECIFICATIONS

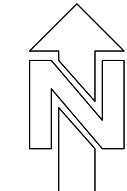
ARMCO BARRIERS TO BE SUPPLIED AND INSTALLED AS DRAWING 182

NOTE:
FOR REFERENCE POINTS REFER TO
TOPO SURVEY DRG NO. CCL-4-7901 500

LOCAL DATUM POINTS		
REF POINT	NORTHINGS [m]	EASTINGS [m]
G18	+44,301	+85,607
G22	-34,863	+85,639

GLOBAL DATUM POINTS			
REF POINT	NORTHINGS [m]	EASTINGS [m]	LEVEL [m]
G18	417762,765	416976,109	58,573
G22	417688,567	417003,706	58,136

DRAWING NO. GB4510174-C-69-0001-299-0087



For slab details see drawing 182
For drainage details see drawing 183
For sumps details see drawing 184
For electrical ducts see drawing 187



SCALE 1:100

APPROX 920m 2 OF NEW CONCRETE HARDSTANDING EXCLUDING RAMP

58.750 PROPOSED FINISHED LEVELS IN BOXES

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Client	PM PROJEN				
Project	SYNGENTA - PHOENIX 2				
Office	LIVERPOOL				
Discipline	CIVIL / STRUCTURAL				
Title	WAREHOUSE / ANNEXE NEW SLAB / DRAINAGE GA				
Drawn	CY	Date	26.5.2022		
Checked	JA	Scale @ A1	1:100		
Approved	CY	Status	CONSTRUCTION		



Job number	Drawing number	Revision
4/7901	181	C1

PART SITE PLAN AS PROPOSED - NEW WAREHOUSE& ANNEXE- 1:100 @ A1

APPENDIX B – MICRODRAINAGE CALCULATIONS

Ramboll UK Ltd		Page 1
240 Blackfriars Road London SE1 8NW	RAINFALL RUNOFF CALCULATION	
Date 05/09/2023 08:26 File	Designed by THILLAS Checked by	
Micro Drainage Source Control 2020.1		
ICP SUDS Mean Annual Flood Input Return Period (years) 100 Soil 0.450 Area (ha) 0.057 Urban 0.000 SAAR (mm) 900 Region Number Region 3 Results 1/s QBAR Rural 0.3 QBAR Urban 0.3 Q100 years 0.7 Q1 year 0.3 Q30 years 0.6 Q100 years 0.7		
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Ramboll UK Ltd			Page 1		
240 Blackfriars Road London SE1 8NW		STORAGE CALCULATION			
Date 05/09/2023 08:41 File		Designed by THILLAS Checked by			
Micro Drainage		Source Control 2020.1			
Summary of Results for 100 year Return Period (+40%)					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	0.367	0.367	0.2	12.9	O K
30 min Summer	0.496	0.496	0.2	17.4	O K
60 min Summer	0.636	0.636	0.2	22.2	O K
120 min Summer	0.778	0.778	0.3	27.2	O K
180 min Summer	0.856	0.856	0.3	30.0	O K
240 min Summer	0.904	0.904	0.3	31.6	O K
360 min Summer	0.965	0.965	0.3	33.8	O K
480 min Summer	1.001	1.001	0.3	35.0	O K
600 min Summer	1.021	1.021	0.3	35.7	O K
720 min Summer	1.030	1.030	0.3	36.1	O K
960 min Summer	1.031	1.031	0.3	36.1	O K
1440 min Summer	1.018	1.018	0.3	35.6	O K
15 min Winter	0.412	0.412	0.2	14.4	O K
30 min Winter	0.556	0.556	0.2	19.5	O K
60 min Winter	0.714	0.714	0.3	25.0	O K
120 min Winter	0.877	0.877	0.3	30.7	O K
180 min Winter	0.966	0.966	0.3	33.8	O K
240 min Winter	1.022	1.022	0.3	35.8	O K
360 min Winter	1.096	1.096	0.3	38.4	O K
480 min Winter	1.140	1.140	0.3	39.9	O K
600 min Winter	1.168	1.168	0.3	40.9	O K
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	122.350	0.0	12.8	27	
30 min Summer	82.996	0.0	15.9	41	
60 min Summer	53.779	0.0	22.9	70	
120 min Summer	33.632	0.0	28.6	130	
180 min Summer	25.157	0.0	32.1	188	
240 min Summer	20.313	0.0	34.4	248	
360 min Summer	15.013	0.0	37.2	366	
480 min Summer	12.102	0.0	38.1	484	
600 min Summer	10.229	0.0	38.7	602	
720 min Summer	8.911	0.0	39.3	722	
960 min Summer	7.160	0.0	40.1	898	
1440 min Summer	5.250	0.0	40.7	1134	
15 min Winter	122.350	0.0	14.2	26	
30 min Winter	82.996	0.0	16.4	41	
60 min Winter	53.779	0.0	25.7	70	
120 min Winter	33.632	0.0	32.0	128	
180 min Winter	25.157	0.0	35.7	186	
240 min Winter	20.313	0.0	37.7	244	
360 min Winter	15.013	0.0	39.2	360	
480 min Winter	12.102	0.0	40.3	474	
600 min Winter	10.229	0.0	41.2	588	
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240 Blackfriars Road London SE1 8NW		STORAGE CALCULATION																																														
Date 05/09/2023 08:41 File		Designed by THILLAS Checked by																																														
Micro Drainage		Source Control 2020.1																																														
Summary of Results for 100 year Return Period (+40%) <table><thead><tr><th>Storm Event</th><th>Max Level (m)</th><th>Max Depth (m)</th><th>Max Control (l/s)</th><th>Max Volume (m³)</th><th>Status</th></tr></thead><tbody><tr><td>720 min Winter</td><td>1.184</td><td>1.184</td><td>0.3</td><td>41.4</td><td>O K</td></tr><tr><td>960 min Winter</td><td>1.195</td><td>1.195</td><td>0.3</td><td>41.8</td><td>O K</td></tr><tr><td>1440 min Winter</td><td>1.175</td><td>1.175</td><td>0.3</td><td>41.1</td><td>O K</td></tr></tbody></table> <table><thead><tr><th>Storm Event</th><th>Rain (mm/hr)</th><th>Flooded Volume (m³)</th><th>Discharge Volume (m³)</th><th>Time-Peak (mins)</th></tr></thead><tbody><tr><td>720 min Winter</td><td>8.911</td><td>0.0</td><td>42.0</td><td>700</td></tr><tr><td>960 min Winter</td><td>7.160</td><td>0.0</td><td>42.9</td><td>920</td></tr><tr><td>1440 min Winter</td><td>5.250</td><td>0.0</td><td>43.3</td><td>1184</td></tr></tbody></table>					Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status	720 min Winter	1.184	1.184	0.3	41.4	O K	960 min Winter	1.195	1.195	0.3	41.8	O K	1440 min Winter	1.175	1.175	0.3	41.1	O K	Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	720 min Winter	8.911	0.0	42.0	700	960 min Winter	7.160	0.0	42.9	920	1440 min Winter	5.250	0.0	43.3	1184
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Ramboll UK Ltd		Page 3
240 Blackfriars Road London SE1 8NW	STORAGE CALCULATION	
Date 05/09/2023 08:41 File	Designed by THILLAS Checked by	
Micro Drainage Source Control 2020.1		

Rainfall Details

Rainfall Model	FSR	Winter Storms	Yes
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	19.000	Shortest Storm (mins)	15
Ratio R	0.332	Longest Storm (mins)	1440
Summer Storms	Yes	Climate Change %	+40

Time Area Diagram

Total Area (ha) 0.057

Time (mins)	Area	Time (mins)	Area	Time (mins)	Area
From: To:	(ha)	From: To:	(ha)	From: To:	(ha)
0 4	0.019	4 8	0.019	8 12	0.019

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Ramboll UK Ltd		Page 4
240 Blackfriars Road London SE1 8NW	STORAGE CALCULATION	
Date 05/09/2023 08:41 File	Designed by THILLAS Checked by	
Micro Drainage Source Control 2020.1		

Model Details

Storage is Online Cover Level (m) 2.200

Tank or Pond Structure

Invert Level (m) 0.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	35.0	0.100	35.0	1.900	35.0	2.000	0.0

Hydro-Brake® Optimum Outflow Control

Unit Reference MD-SHE-0024-3000-1000-3000

Design Head (m) 1.000

Design Flow (l/s) 0.3

Flush-Flo™ Calculated

Objective Minimise upstream storage

Application Surface

Sump Available Yes

Diameter (mm) 24

Invert Level (m) 0.000

Minimum Outlet Pipe Diameter (mm) 75

Suggested Manhole Diameter (mm) 1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.000	0.3
Flush-Flo™	0.106	0.2
Kick-Flo®	0.218	0.2
Mean Flow over Head Range	-	0.2

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	0.2	1.200	0.3	3.000	0.5	7.000	0.7
0.200	0.2	1.400	0.3	3.500	0.5	7.500	0.7
0.300	0.2	1.600	0.4	4.000	0.5	8.000	0.7
0.400	0.2	1.800	0.4	4.500	0.6	8.500	0.8
0.500	0.2	2.000	0.4	5.000	0.6	9.000	0.8
0.600	0.2	2.200	0.4	5.500	0.6	9.500	0.8
0.800	0.3	2.400	0.4	6.000	0.7		
1.000	0.3	2.600	0.5	6.500	0.7		

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